ICC-ES Evaluation Report

ESR-2604

Reissued January 2026

Revised July 30, 2026

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This report also contains:

- City of LA Supplement
- FL Supplement

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23.30—Connectors - General	REPORT HOLDER: SIMPSON STRONG-TIE COMPANY INC.	EVALUATION SUBJECT: SIMPSON STRONG-TIE® COLUMN CAPS AND POST CAPS FOR WOOD CONSTRUCTION	

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2024, 2021, 2018, 2015, and 2012 *International Building Code®* (IBC)
- 2024, 2021, 2018, 2015, and 2012 *International Residential Code®* (IRC)

Property evaluated:

Structural

2.0 USES

The Simpson Strong-Tie® CC/ECC column caps (Table 1) and the CCQ/ECCQ column caps (Table 2) are used to connect wood beams to wood posts and timbers in engineered applications in accordance with Section 2304.10.4 of the 2024 and 2021 IBC, Section 2304.10.3 of the 2018 and 2015 IBC, Section 2304.9.3 of the 2012 and 2009 IBC and Section R301.1.3 of the IRC. The AC/ACE/ACH post caps (Table 3), LPC post caps (Table 4), PC/EPC post caps (Table 5), and the BC/BCS post caps (Table 6) are used to provide a positive connection between post-and-beam construction used to support wood framing members to resist uplift forces and lateral displacement of the beam in accordance with Section 2304.10.8 of the 2024 and 2021 IBC, Section 2304.10.7 of the 2018 and 2015 IBC, Section 2304.9.7 of the 2012 and 2009 IBC and Section R502.9 of the IRC. The MPSC column connection (Table 7) is used to connect glulam to glulam columns in a mass timber point supported system together. The MPSC column connection is also used to connect structural composite lumber cross-laminated timber (SCL CLT) column to SCL CLT column and panel [CLT or Mass Plywood panel (MPP)] in a mass timber point supported system together.

3.0 DESCRIPTION

3.1 General–Column Caps and Post Caps

3.1.1 CC/ECC Column Caps: The CC3$^1/_4$, ECC3$^1/_4$, CC4, ECC4, CC4.62, ECC4.62, CC6 and ECC6 are fabricated from two No. 7 gage steel straps factory welded to a No. 7 gage steel U-shaped channel, where $^3/_{16}$-inch-thick (4.8 mm) by 2$^1/_2$-inch-long (63.5 mm) fillet welds are located on one side of each strap attached to the U-channel. All other CC/ECC column caps are fabricated from two No. 3 gage steel straps factory welded to a No. 3 gage steel U-shaped channel, where $^1/_4$-inch-thick (6.4 mm) by 2$^1/_2$-inch-long (63.5 mm) fillet welds

are located on one side of each strap attached to the U-channel. Column caps with fillet welds on both sides of each strap are available. The ECC column caps are designed for use at beam ends. See Table 1 for column cap models, dimensions, required quantity and diameter of bolts, and allowable loads. See Figure 1 for a drawing of a typical CC column cap connector and the ECC44 end column connector.

3.1.2 CCQ/ECCQ Quick Drive Column Caps: The CCQ3, ECCQ3, CCQ4, ECCQ4, CCQ4.62, ECCQ4.62, CCQ6, and ECCQ6 are fabricated from two No. 7 gage steel straps factory welded to a No. 7 gage steel U-shaped channel, where $^{3}/_{16}$-inch-thick (4.8 mm) by $2^{1}/_{2}$-inch-long (63.5 mm) fillet welds are located on one side of each strap attached to the U-channel. The CCQ5, ECCQ5, CCQ7, ECCQ7, CCQ7.1, ECCQ7.1, CCQ8, ECCQ8, CCQ9, ECCQ9, CC10, and ECCQ10 column caps are fabricated from two No. 3 gage steel straps factory welded to a No. 3 gage steel U-shaped channel, where $^{1}/_{4}$-inch-thick (6.4 mm) by $2^{1}/_{2}$-inch-long (63.5 mm) fillet welds are located on one side of each strap attached to the U-channel. Column caps with fillet welds on both sides of each strap are available. The ECCQ column caps are used to connect the end of a beam to a post. See Table 2 for column cap models, dimensions, required quantity of SDS $^{1}/_{4}$ x $2^{1}/_{2}$ screws, and allowable loads. See Figure 2 for a drawing of a CCQ46-SDS2.5 column cap, a typical installation of a CCQ46-SDS2.5 column cap, and of an ECCQ46-SDS2.5 end column cap connector.

3.1.3 AC/ACE/ACH Post Caps: The AC and ACE are two-piece post caps fabricated from No. 18 gage galvanized steel. The ACH are two-piece post caps fabricated from No. 16 gage galvanized steel. The AC/ACE/ACH post caps must be used in pairs and in locations where the supported beam is continuous over the wood post. ACE post caps are used to connect the end of a beam to a post. The ACH can be bent to connect the end of a beam to a post. See Table 3 for dimensions, minimum (MIN) and maximum (MAX) fastener schedules, and allowable uplift and lateral loads corresponding to the minimum and maximum fastener schedules. See Figure 3a for a drawing of a typical AC post cap, Figure 3b for a drawing of a typical installation of an ACE post cap showing the “left” and “right” pieces of the post cap assembly, Figure 3c for a drawing of a typical installation of an ACH and Figure 3d for a typical installation of an ACH installed at the end of a beam.

3.1.4 LPC Light Post Caps: The LPC6 and LPC4 post caps are two-piece post caps fabricated from No. 16 gage and No. 18 gage galvanized steel respectively. The LPC post caps must be used in pairs and in locations where the supported beam is continuous over the wood post. The LPC post caps are designed to be used with wood beams having a width less than the supporting wood post, and can connect continuous beams or the end of beams to a post provided the required nails are installed. Both LPC post caps described in this report have a model designation ending with the letter Z, indicating they have a G185 zinc coating in accordance with ASTM A653. See Table 4 for the connector width for the supporting wood post, required fasteners, and allowable uplift and lateral loads. See Figure 4 for a drawing of a typical LPC post cap connector, and a typical installation where the supported wood beam is continuous over the wood post.

3.1.5 Post Caps: The PC/EPC and PCZ/EPCZ post caps are one-piece connectors. PC44-16 (EPC44-16), PC46-16 (EPC46-16), PC48-16 (EPC48-16), PC64-16 (EPC64-16), PC66-16 (EPC66-16), PC4Z (EPC4Z), PC4RZ (EPC4RZ), PC6Z (EPC6Z), PC6RZ (EPC6RZ), PC8Z (EPC8Z), and the PC8RZ (EPC8RZ) are fabricated from No.16 gage galvanized steel. The PC and PCZ are designed to connect a beam to a post and the EPC and EPCZ are designed to connect the end of a beam to a post. See Table 5 for model numbers, post cap dimensions, fastener options and allowable uplift and lateral loads. See Figure 5 for drawings of typical PC/EPC and PCZ/EPCZ post caps and typical installation drawings.

3.1.6 BC/BCS Post Caps: The BC/BCS post caps are one-piece connectors fabricated from No. 18 gage galvanized steel. The BCS2-2/4 post cap is designed for the connection of double 2x’s to a nominally 4-inch-wide post, and the BCS2-3/6 post cap is designed for the connection of triple 2x’s to a nominally 6-inch-wide post. The BC/BCS post caps are designed to be used with built-up wood beams having a width less than the post width, and can connect continuous beams or the end of beams to a post provided the required nails are installed. See Table 6 for model numbers, post cap dimensions, required fasteners, and allowable uplift and lateral loads. See Figure 6 for drawings of a BC4 post cap and a BCS2-2/4 post cap, and a drawing of a typical installation of a BCS2-2/4 post cap.

3.1.7 MPSC Mass Timber Point Supported Column Connection: The MPSC column connection consists of three primary components. The first component consists of a steel bottom plate welded to a hollow steel section (HSS) tube with a slot cut in it. The second component is a steel top plate welded to knife plate with a slotted hole for all models except the MPSC100-U has a standard bolt hole. The second component is nested into the first component and connected with a singular bolt. See Table 7 for MPSC dimensions, and Table 8 for MPSC allowable loads. See Figure 7 for graphical depiction and typical installation for the MPSC and Figure

8 for MPSC loading Diagram. The final component is a tie plate that provides connection between the MPSC column connection and the surrounding CLT/MPP panels. See Table 9 for tie plate dimensions and allowable lateral load, and Figure 9 for graphical depiction and typical installation for the tie plate.

3.2 Materials:

3.2.1 Steel: The galvanized connectors described in this report are manufactured from galvanized sheet steel complying with ASTM A653, SS designation, Grade 33, with a minimum specified yield strength, F_y, of 33 ksi and tensile strength, F_u, of 45 ksi. The CC/ECC and CCQ/ECCQ column cap connectors and MPSC tie plate are manufactured from steel complying with ASTM A1011, SS designation, Grade 33, with minimum specified yield strength, F_y, of 33 ksi and tensile strength, F_u, of 52 ksi. The CC/ECC and CCQ/ECCQ column cap connectors have a painted finish. The MPSC series column connection is manufactured from five nongalvanized steel components: top and bottom plates, knife plate, HSS tube standoff, and a tie plate. The MPSC top and bottom plates and knife plate are manufactured from steel complying with ASTM A572, Grade 50, with a minimum yield strength, F_y, of 50 ksi (345 MPa) and a minimum tensile strength, F_u, of 65 ksi (448 MPa). The rectangular HSS component of the MPSC column connection is manufactured from steel complying with ASTM A500 Grade C, with a minimum yield strength, F_y, of 50 ksi (345 MPa), and a minimum tensile strength, F_u, of 62 ksi (427 MPa). The MPSC components are either painted or powder coated. Base-metal thicknesses for the connectors in this report are as follows:

NOMINAL THICKNESS (Gage)	MINIMUM BASE METAL THICKNESS (inches)
No. 3[1]	0.2285
No. 7[1]	0.1705
No. 16[2]	0.0555
No. 18[2]	0.0445

For **SI:** 1 inch = 25.4 mm.

[1]Base-metal thickness for steel conforming to ASTM A1011.
[2]Base-metal thickness for steel conforming to ASTM A653.

Some connectors (designated with a model number ending with Z) are available with a G185 zinc coating specification in accordance with ASTM A653. Some models (designated with a model number ending with HDG) are available with a hot-dip galvanization, also known as "batch" galvanization, in accordance with ASTM A123, with a minimum specified coating weight of 2.0 ounces of zinc per square foot of surface area (610 g/m^2), total for both sides. Model numbers in this report do not include the Z or HDG ending (except for Table 4), but the information shown applies. The lumber treater and the holder of this report (Simpson Strong-Tie Company) should be contacted for recommendations on the appropriate coating or material to specify for use of the steel connectors in contact with the specific proprietary preservative treated or fire retardant treated lumber.

3.2.2 Wood: Wood members used with column cap and post cap connectors (excluding MPSC column connection) must be either sawn lumber or engineered lumber having a minimum specific gravity of 0.50 (minimum equivalent specific gravity of 0.50 for engineered lumber), and having a maximum moisture content of 19 percent (16 percent for engineered lumber) except as noted in Section 4.1. The thickness of the supporting wood member must be equal to or greater than the length of the fasteners specified in the tables in this report, or as required by wood member design, whichever is greater. For installation in engineered wood members, minimum allowable nail spacing and end and edge distances, as specified in the applicable evaluation report for the engineered wood product, must be met.

Wood column members used with the MPSC column connection must be either glued laminated timber (glulam) or structural composite lumber cross-laminated timber (SCL CLT) with the minimum specifications provided in Table 8. SCL CLT columns must be specified and properties confirmed by the registered design professional. The glued laminated timber must have four or more laminations, stressed primarily in axial tension or compression and have an assigned specific gravity, SG_{NDS}, of 0.50 or greater, complying with Table 5B of the NDS Supplement. The applicable glulam wood species are shown in Tables 7 and Table 8 of this report.

The wood roof and floor panels used with the MPSC column connection must be either sawn lumber CLT with a minimum equivalent specific gravity of 0.42 and species of Spruce Pine Fir (SPF) and/or Douglas Fir

(DF) or SCL CLT MPP Grade F10 or better complying with ESR-4760 and a minimum equivalent specific gravity 0.5.

3.2.3 Fasteners: Nails used with connectors described in this report must be bright or hot-dipped galvanized carbon steel nails complying with ASTM F1667 with minimum fastener dimensions and bending yield strengths (F_{yb}) shown in the following table. Alternatively, nails of other materials or finishes may be used when they are recognized in an ICC-ES evaluation report as having bending yield strength and withdrawal capacity equal to or better than those of a bright carbon steel nail of the same nominal diameter.

FASTENER	SHANK DIAMETER (inches)	NAIL LENGTH (inches)	F_{yb} (psi)
10d	0.148	3	90,000
16d	0.162	$3^1/_2$	90,000

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.895 kPa.

For the CC/ECC/ECCU connectors, bolts are to be Standard Hex Bolts complying with ANSI/ASME B18.2.1, and, at a minimum, must comply with ASTM A36 or A307.

SDS screws are Simpson Strong-Tie® Strong-Drive® screws that comply with ESR-2236. SDS screws, where required in the report, are provided with the connector, except for the MPSC column connection. SD10212 screws are Simpson Strong-Tie® Strong-Drive® screws that comply with ESR-3046. SDCF screws are Simpson Strong-Tie® Strong-Drive® SDCF TIMBER-CF screws that comply with ESR-3046. SDCF screws, where required in the report, are not provided with the connector.

For the MPSC Column Connection, bolts are heavy hex bolts complying with ANSI/ASME B18.2.6, and, at a minimum, must comply with ASTM F3125 A325 Type 1. The bolts described in Table 7 and Table 8 must have a diameter of 0.75 inch (19.05 mm).

Nails used in contact with preservative-treated or fire-retardant-treated lumber must be hot-dipped galvanized carbon steel nails. Alternatively, nails of other materials or finishes may be used when they are recognized in an ICC-ES evaluation report for use in the applicable treated lumber.

Bolts used in contact with preservative treated or fire retardant treated lumber must comply with Section 2304.10.6 of the 2024 and 2021 IBC, Section 2304.10.5 of the 2018 and 2015 IBC, Section 2304.9.5 of the 2012 and 2009 IBC, Section R304.3 of the 2024 IRC or Section R317.3 of the 2021, 2018, 2015 and 2012 IRC, as applicable. SDS screws used in contact with preservative-treated or fire-retardant-treated lumber must, as a minimum, comply with ESR-2236. For use with treated lumber, the lumber treater or this report holder (Simpson Strong-Tie Company), or both, should be contacted for recommendations on the appropriate coating or material to specify for the fasteners as well as the connection capacities of fasteners used with the specific proprietary preservative treated or fire retardant treated lumber. Nails and bolts required in this evaluation report are not provided with the connectors.

4.0 DESIGN AND INSTALLATION

4.1 Design:

The tabulated allowable loads shown in the product tables of this report are based on Allowable Stress Design (ASD) and include the load duration factor, C_D, corresponding with the applicable loads in accordance with the *National Design Specification® for Wood Construction* (NDS and its Supplement).

Tabulated allowable loads apply to products connected to wood used under dry conditions and where sustained temperatures are 100°F (37.8°C) or less. When products are installed to wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where wet service is expected, the allowable loads must be adjusted by the wet service factor, C_M, specified in the NDS for dowel-type fasteners. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding 100°F (37.8°C), the allowable loads in this report must be adjusted by the applicable temperature factor, C_t, specified in the NDS. Connected wood members must be analyzed for load-carrying capacity at the connection in accordance with the NDS.

4.2 Installation:

Installation of the connectors must be in accordance with this evaluation report and the manufacturer's published installation instructions. In the event of a conflict between this report and the manufacturer's published installation instructions, this report governs.

5.0 CONDITIONS OF USE:

The Simpson Strong-Tie connectors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 The connectors must be manufactured, identified and installed in accordance with this report and the manufacturer's published installation instructions. A copy of the instructions must be available at the jobsite at all times during installation.

5.2 Calculations showing compliance with this report must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed

5.3 Adjustment factors noted in Section 4.1 and the applicable codes must be considered, where applicable.

5.4 Connected wood members and fasteners must comply, respectively, with Sections 3.2.2 and 3.2.3 of this report.

5.5 Use of connectors with preservative treated or fire retardant treated lumber must be in accordance with Section 3.2.1 of this report. Use of fasteners with preservative treated or fire retardant treated lumber must be in accordance with Section 3.2.3 of this report.

5.6 Welded connectors are manufactured under a quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices (AC13), dated April 2024.

7.0 IDENTIFICATION

7.1 The ICC-ES mark of conformity, electronic labeling or the evaluation report number (ICC-ES ESR-2604) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.

7.2 In addition, the products described in this report are identified with a die-stamped label or an adhesive label indicating the name of the manufacturer (Simpson Strong-Tie), the model number, and the number of an index evaluation report (ESR-2523) that is used as an identifier for the products recognized in this report. Additionally, the factory-welded connectors manufactured in the United States and Canada are identified with the acronym of the inspection agency (ICC-ES).

7.3 The report holder's contact information is the following:

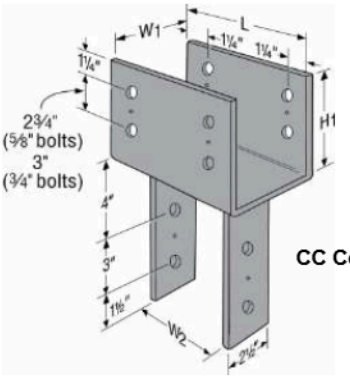
SIMPSON STRONG-TIE COMPANY INC.
5956 WEST LAS POSITAS BOULEVARD
PLEASANTON, CALIFORNIA 94588
(800) 925-5099
www.strongtie.com

TABLE 1—CC AND ECC SERIES COLUMN CAPS

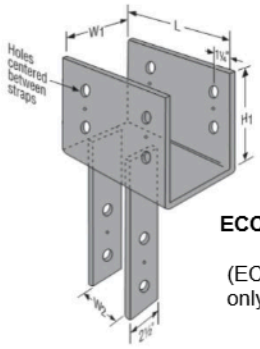
MODEL NO.	Width for Beam (W_1) (in.)	Width for Post (W_2) (in.)	COLUMN CAP DIMENSIONS (inches)				BOLTS[1]					ALLOWABLE LOADS[2,3,4] (lbs)			
			Bearing Length for Beam (L)			U-Channel Height for Beam (H_1)	Size (in.)	Quantity				CC/ECCU	CC	ECC	ECCU
								Beam			Post	Uplift[5,6,7,11]	Download[8,9,10]		
			CC	ECC	ECCU			CC	ECC	ECCU		C_D = 1.6	C_D = 1.0		
CC3 1/4-4	3 1/4	3 5/8	11	7 1/2	9 1/2	6 1/2	5/8	4	2	4	2	3,150	16,980	6,835	6,835
CC3 1/4-6		5 1/2	11	7 1/2	9 1/2	6 1/2	5/8	4	2	4	2	3,150	21,485	10,740	10,740
CC44	3 5/8	3 5/8	7	5 1/2	6 1/2	4	5/8	2	1	4	2	1,850	19,020	7,655	7,655
CC46		5 1/2	11	8 1/2	9 1/2	6 1/2	5/8	4	2	4	2	3,530	24,065	12,030	12,030
CC48		7 1/2	11	8 1/2	9 1/2	6 1/2	5/8	4	2	4	2	3,530	24,065	16,405	16,405
CC4.62-3.62	4 5/8	3 5/8	11	8 1/2	9 1/2	6 1/2	5/8	4	2	4	2	4,535	23,390	9,845	9,845
CC4.62-4.62		4 5/8	11	8 1/2	9 1/2	6 1/2	5/8	4	2	4	2	4,535	30,070	12,655	12,655
CC4.62-5.50		5 1/2	11	8 1/2	9 1/2	6 1/2	5/8	4	2	4	2	4,535	30,940	15,470	15,470
CC5 1/4-4	5 1/4	3 5/8	13	9 1/2	10 1/2	8	3/4	4	2	4	2	6,300	26,635	11,210	11,210
CC5 1/4-6		5 1/2	13	9 1/2	10 1/2	8	3/4	4	2	4	2	6,500	28,190	17,615	17,615
CC5 1/4-8		7 1/2	13	9 1/2	10 1/2	8	3/4	4	2	4	2	6,645	35,235	24,025	24,025
CC64	5 1/2	3 5/8	11	7 1/2	9 1/2	6 1/2	5/8	4	2	4	2	5,545	28,585	12,030	12,030
CC66		5 1/2	11	7 1/2	9 1/2	6 1/2	5/8	4	2	4	2	5,545	33,275	18,905	18,905
CC68		7 1/2	11	9 1/2	9 1/2	6 1/2	5/8	4	2	4	2	5,545	37,815	25,780	25,780
CC6-7 1/8		7 1/8	11	9 1/2	9 1/2	6 1/2	5/8	4	2	4	2	5,545	37,815	24,490	24,490
CC74	6 7/8	3 5/8	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,330	33,490	15,355	15,355
CC76		5 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,790	37,125	24,130	24,130
CC77		6 7/8	13	10 1/2	10 1/2	8	3/4	4	2	4	2	7,020	48,265	29,615	29,615
CC78		7 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	7,145	48,265	32,090	32,905
CC7 1/8-4	7 1/8	3 5/8	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,360	34,730	18,375	18,375
CC7 1/8-6		5 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,825	38,500	28,875	28,875
CC7 1/8-7 1/8		7 1/8	13	10 1/2	10 1/2	8	3/4	4	2	4	2	7,105	57,750	36,750	36,750
CC7 1/8-8		7 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	7,190	52,500	39,375	39,375
CC84		3 5/8	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,410	37,210	16,405	16,405
CC86		5 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	6,885	41,250	25,780	25,780
CC88		7 1/2	13	10 1/2	10 1/2	8	3/4	4	2	4	2	7,250	51,565	35,155	35,155
CC94	8 7/8	3 5/8	13	10 1/2	10 1/2	8	3/4	4	4	4	2	6,580	47,545	19,905	19,905
CC96		5 1/2	13	10 1/2	10 1/2	8	3/4	4	4	4	2	7,080	48,125	31,280	31,280
CC98		7 1/2	13	10 1/2	10 1/2	8	3/4	4	4	4	2	7,455	62,565	42,655	42,655
CC106	9 1/2	5 1/2	13	10 1/2	10 1/2	8	3/4	4	4	4	2	7,160	52,250	32,655	32,655

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

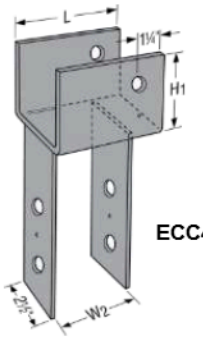
[1]ECC has one-half the tabulated beam bolts. ECC9 and ECC10 have four beam bolts. Bolt holes bored into the wood beam and post must be no less than 1/32 inch greater and no more than 1/16 inch greater than the diameter of the bolt. See Section 3.2.3. (Bolts are not provided with the CC/ECC/ECCU connectors.)
[2]Tabulated allowable load must be selected based on duration of load as permitted by the applicable building code.
[3]The wood post depth must be equal to the wood beam width (W_1).
[4]If structural composite lumber posts are used, installation of the fasteners into the wide face (fasteners perpendicular to the strands/veneers) is required in order to obtain the loads listed in this report. The structural composite lumber must have an ICC-ES report that shows fastener design specific gravity equivalent of 0.50 or better.
[5]The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable loads must be reduced when other load durations govern.
[6]Allowable uplift loads for the CC column caps do not apply to spliced beam conditions.
[7]Allowable uplift loads assume a beam height of 11 inches to ensure minimum edge distance for the top bolts in the U-shaped channel loaded perpendicular to the grain of the wood beam.
[8]Allowable downloads are for beams that are continuous over the length (L) of the CC connector.
[9]When a spliced beam condition occurs, that is, where the ends of two beams are supported by the post and connected to the CC post cap connector, the splice must occur at the middle of the connector and the maximum allowable download for each spliced beam is one half of the tabulated allowable download. When spliced beams must be connected together to transfer design tension loads (i.e., lateral loads parallel to the beams), the connection must be by means other than the column cap.
[10]Tabulated allowable download is based on the lesser capacity determined from compression-perpendicular-to-grain stress ($F_{c\perp}$) for the supported beam of 560 psi for glulam and 625 psi for DF-L sawn lumber, as applicable, and the following compression-parallel-to-grain stress ($F_{c\parallel}$) for the wood post: 1,650 psi for glulam, 1,350 psi for 4-inch-wide (W_2 dimension) sawn lumber, and 1,000 psi for 5-inch-wide and wider (W_2 dimension) sawn lumber, as applicable. Allowable loads may not be increased for short-term loading.
[11]Allowable uplift loads require the supported beam member having a minimum shear parallel-to-grain, F_v, of 180 psi, such as for DF-L sawn lumber.



CC Column Cap



ECCU Column Cap
(ECCU44 has only 2 bolts to beam)



ECC44 Column Cap

FIGURE 1—CC AND ECC COLUMN CAPS

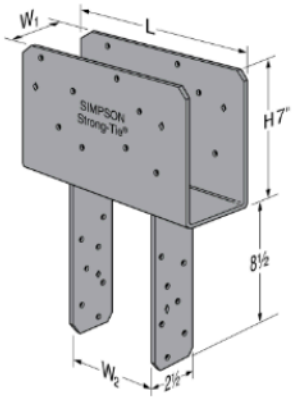
TABLE 2—CCQ AND ECCQ SERIES COLUMN CAPS

MODEL NO.	COLUMN CAP DIMENSIONS (inches)					QUANTITY OF SDS¼ x 2½ SCREWS[1]		ALLOWABLE LOADS[2,3,4,5] (lbs)			
	Width for Beam (W_1) (in.)	Width for Post (W_2) (in.)	Bearing Length for Beam (L) (in.)		U-Channel Height for Beam (H_1) (in.)			CCQ	ECCQ[6]	CCQ	ECCQ[6]
			CCQ	ECCQ		Into Beam	Into Post	Uplift[6,7,8]		Download[9,10,11,12]	
								C_D=1.6	C_D=1.6	C_D=1.0	
CCQ3-4SDS2.5	3¼	3⅝	11	8½	7	16	14	5,370	3,465	16,980	6,125
CCQ3-6SDS2.5		5½						5,370	3,465	21,485	10,740
CCQ44SDS2.5	3⅝	3⅝	11	8½	7	16	14	5,370	3,785	19,020	7,655
CCQ46SDS2.5		5½						6,785	3,785	24,065	12,030
CCQ48SDS2.5		7½						6,785	3,785	24,065	16,405
CCQ4.62-3.62SDS	4⅝	3⅝	11	8½	7	16	14	5,370	3,785	23,390	9,845
CCQ4.62-4.62SDS		4⅝						5,370	3,785	30,070	12,655
CCQ4.62-5.50SDS		5½						6,785	3,785	30,940	15,470
CCQ5-4SDS2.5	5¼	3⅝	11	8½	7	16	14	5,370	4,040	26,635	11,210
CCQ5-6SDS2.5		5½						6,785	5,355	28,190	17,615
CCQ5-8SDS2.5		7½						6,785	5,355	35,235	24,025
CCQ64SDS2.5	5½	3⅝	11	8½	7	16	14	5,370	3,785	28,585	12,030
CCQ66SDS2.5		5½						6,785	3,785	33,275	18,905
CCQ68SDS2.5		7½						6,785	3,785	37,815	25,780
CCQ6-7.13SDS2.5		7⅛						6,785	3,785	37,815	24,490
CCQ74SDS2.5	6⅞	3⅝	11	8½	7	16	14	5,370	4,040	33,490	15,355
CCQ76SDS2.5		5½						6,785	5,355	37,125	24,130
CCQ77SDS2.5		6⅞						6,785	5,355	48,265	29,615
CCQ78SDS2.5		7½						6,785	5,355	48,265	32,905
CCQ7.1-4SDS2.5	7⅛	3⅝	11	8½	7	16	14	5,370	4,040	34,730	18,375
CCQ7.1-6SDS2.5		5½						6,785	5,355	38,500	28,875
CCQ7.1-7SDS2.5		7⅛						6,785	5,355	57,750	36,750
CCQ7.1-8SDS2.5		7½						6,785	5,355	52,500	39,375
CCQ84SDS2.5	7½	3⅝	11	8½	7	16	14	6,785	5,355	37,210	16,405
CCQ86SDS2.5		5½						6,785	5,355	41,250	25,780
CCQ88SDS2.5		7½						6,785	5,355	51,565	35,155
CCQ94SDS2.5	8⅞	3⅝	11	8½	7	16	14	6,785	5,355	47,545	19,905
CCQ96SDS2.5		5½						6,785	5,355	48,125	31,280
CCQ98SDS2.5		7½						6,785	5,355	62,565	42,655
CCQ106SDS2.5	9½	5½	11	8½	7	16	14	6,785	5,355	52,250	32,655

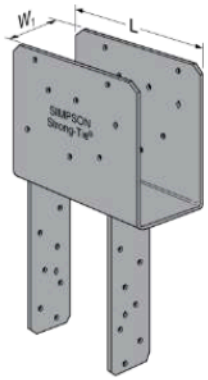
For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

[1]The model number for the SDS¼x2½ is SDS25250 (see ESR-2236). The screws are included as components of the CCQ/ECCQ column caps.
[2]Tabulated allowable load must be selected based on duration of load as permitted by the applicable building code.
[3]The wood post depth must be equal to the wood beam width (W_1).
[4]If structural composite lumber posts are used, installation of the fasteners into the wide face (fasteners perpendicular to the strands/veneers) is required in order to obtain the loads listed in this report. The structural composite lumber must have an ICC-ES report that shows fastener design specific gravity equivalent of 0.50 or better.

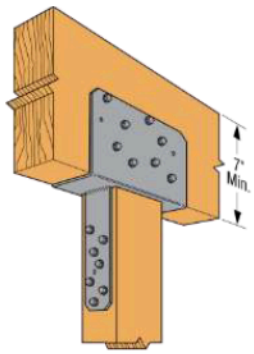
[5]ECCQ uses 14–SDS $^1/_4$ x $2^1/_2$ screws into the beam and 14–SDS $^1/_4$ x $2^1/_2$ screws into the post.
[6]The uplift loads have been increased for wind or earthquake loading with no further increase allowed. The allowable loads must be reduced when other load durations govern.
[7]Allowable uplift loads for the CCQ column caps do not apply to spliced beam conditions.
[8]Allowable uplift loads assume a minimum beam height of 7 inches to ensure minimum edge distance for the top SDS screws in the U-shaped channel loaded perpendicular to the grain of the wood beam.
[9]Allowable downloads for beams that are continuous over the length (L) of the CCQ connector.
[10]When a spliced beam condition occurs, that is, where the ends of two beams are supported by the post and connected to the CCQ post cap connector, the splice must occur at the middle of the connector and the maximum allowable download for each spliced beam is one half of the tabulated allowable download. When spliced beams must be connected together to transfer design tension loads (i.e., lateral loads parallel to the beams), the connection must be by means other than the column cap.
[11]Tabulated allowable download is based on the lesser capacity determined from compression-perpendicular-to-grain stress ($F_{c\perp}$) for the supported beam of 560 psi for glulam and 625 psi for DF-L sawn lumber, as applicable, and the following compression-parallel-to-grain stress ($F_{c||}$) for the wood post: 1,550 psi for glulam, 1,350 psi for 4-inch-wide (W_2 dimension) sawn lumber, and 1,000 psi for 5-inch-wide and wider (W_2 dimension) sawn lumber, as applicable.
[12]Allowable downloads may not be increased for short-term loading.



CCQ46SDS2.5 Column Cap



ECCQ46SDS2.5 Column Cap for End Conditions



CCQ46SDS2.5 Installation

FIGURE 2—CCQ AND ECCQ COLUMN CAPS

TABLE 3—AC, ACE, AND ACH POST CAPS

MODEL NO. [1,2]		POST CAP DIMENSIONS (inches)		NAILS (Quantity–Type)		ALLOWABLE LOADS[3,4,5] (lbs)	
						Uplift [6]	Lateral [7]
		W	L	Into the Beam	Into the Post	C_D = 1.6	C_D = 1.6
AC4	MIN	3 9/16	6 1/2	8 – 16d	8 – 16d	1,745	1,610
	MAX			14 – 16d	14 – 16d	2,490	1,475
AC4R	MIN	4	7	8 – 16d	8 – 16d	1,745	1,610
	MAX			14 – 16d	14 – 16d	2,490	2,075
ACE4	MIN	—	4 1/2	6 – 16d	6 – 16d	1,235	750
	MAX			10 – 16d	10 – 16d	1,950	1,265
ACH4Z	-	3 9/16	6 1/2	20-16dx2 1/2	20-16dx2 1/2	4,045	1,765
	-			20-SD10212	20-SD10212	5,895	2,595
ACH4Z (END)	-	3 9/16	6 1/2	20-16dx2 1/2	20-16dx2 1/2	2,580	1,360
	-			20-SD10212	20-SD10212	2,680	1,815
AC6	MIN	5 1/2	8 1/2	8 – 16d	8 – 16d	1,665	1,565
	MAX			14 – 16d	14 – 16d	2,815	2,075
AC6R	MIN	6	9	8 – 16d	8 – 16d	1,665	1,565
	MAX			14 – 16d	14 – 16d	3,055	2,450
ACE6	MIN	—	6 1/2	6 – 16d	6 – 16d	1,235	835
	MAX			10 – 16d	10 – 16d	1,950	1,760
ACH6Z	-	5 1/2	8 1/2	20-16dx2 1/2	20-16dx2 1/2	4,045	2,640
	-			20-SD10212	20-SD10212	5,895	4,130
ACH6Z (END)	-	5 1/2	8 1/2	20-16dx2 1/2	20-16dx2 1/2	2,580	1,965
	-			20-SD10212	20-SD10212	2,680	2,200

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

[1]"MIN" suffix to the model No. indicates that only the round holes must be filled with the quantity and type of nails specified in the table to achieve the tabulated allowable load values.

[2]"MAX" suffix to the model No. indicates that both round and triangular holes must be filled with the quantity of nails specified in the table to achieve the tabulated allowable load values.

[3]The allowable uplift loads do not apply to splice conditions. When a spliced beam condition occurs, that is, where the ends of two beams are supported by the wood post and connected to the AC post cap connector, the condition must be designed and detailed to transfer the tension (lateral) loads by means other than the post cap.

[4]Allowable uplift and lateral loads apply only for AC, ACE and ACH post cap connectors installed in pairs, as shown in Figure 3b, Figure 3c and Figure 3d with each piece connected to the wood post and beam with an equal amount and type of fasteners.

[5]Allowable uplift and lateral loads have been increased for wind or earthquake loading with no further increase allowed. The allowable loads must be reduced when other load durations govern.

[6]Allowable lateral loads are parallel to the length of the supported wood beam, as shown in Figure 3b.

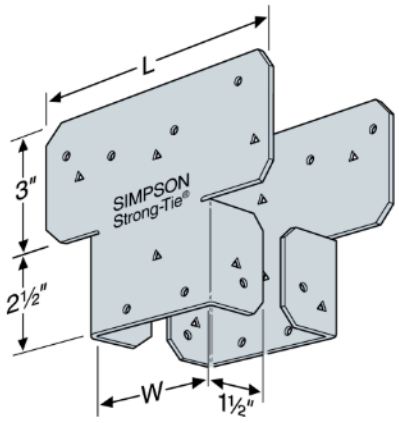


FIGURE 3a—AC POST CAP CONNECTOR COMPONENTS

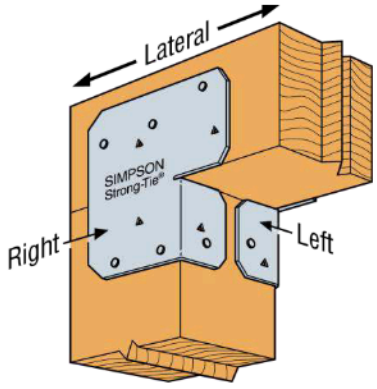


FIGURE 3b—TYPICAL ACE INSTALLATION

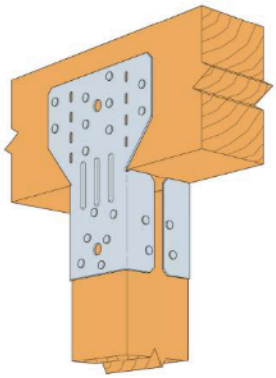


FIGURE 3c TYPICAL ACH INSTALLATION

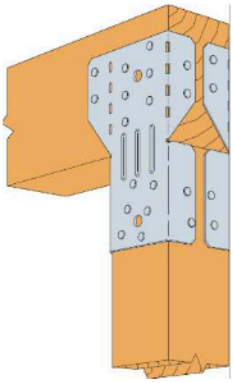


FIGURE 3d TYPICAL ACH (END) INSTALLATION

FIGURE 3—AC, ACE AND ACH POST CAPS

TABLE 4—LPC LIGHT POST CAPS[1,2]

MODEL NO.	CONNECTOR WIDTH FOR WOOD POST (inches)	NAILS (Quantity-Type)		ALLOWABLE LOADS[3,4,5] (lbs.)	
				Uplift[6]	Lateral[7]
		Into the Beam	Into the Post	C_D = 1.6	C_D = 1.6
LPC4Z	$3^{9}/_{16}$	8 –10d	8 –10d	755	760
LPC6Z	$5^{9}/_{16}$	8 –10d	8 –10d	920	885

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

[1]The LPCZ post cap is a two-piece wood-beam-to-post connector that is designed to be used with wood beams having a width less than the post width, as implied in Figure 4.
[2]The LPC4 and LPC6 models shown in the table end with the letter Z, indicating that they have a G185 zinc coating in accordance with ASTM A653.
[3]The allowable uplift loads do not apply to spliced beams, that is, where the ends of two beams are supported by the wood post and connected to the LPC post cap connector. A spliced beam condition must be designed and detailed to transfer the tension load (i.e., tabulated allowable lateral loads) by means other than the column cap.
[4]Allowable uplift and lateral loads apply only for LPC post cap connectors installed in pairs, as shown in Figure 4, with each piece connected to the wood post and beam with an equal amount and type of nails.
[5]Allowable uplift and lateral loads have been increased for wind or earthquake loading with no further increase allowed. The allowable loads must be reduced when other load durations govern.
[6]Allowable uplift loads for the LPC column caps do not apply to spliced beam conditions.
[7]Allowable lateral loads are parallel to the length of the supported wood beam, as shown in Figure 4.

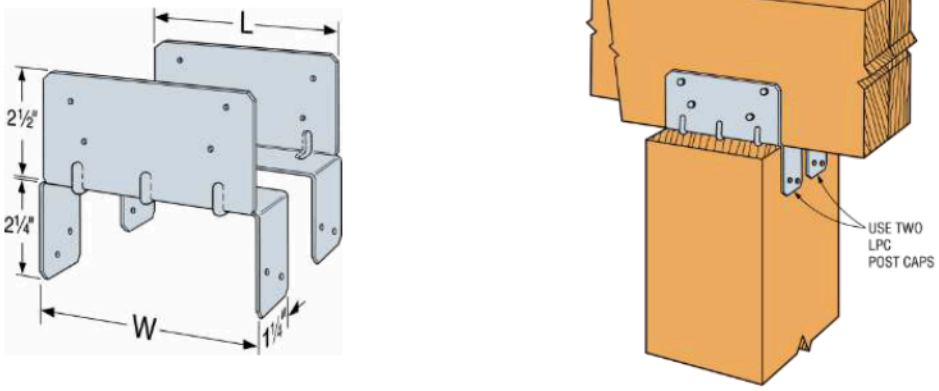


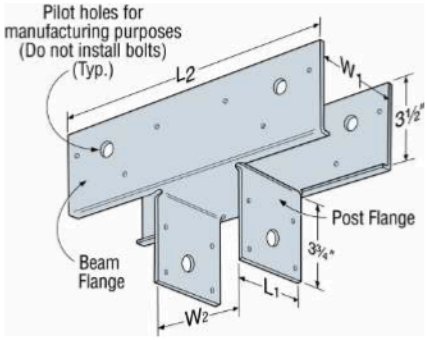
FIGURE 4—LPCZ POST CAPS

TABLE 5—PC/EPC & PCZ/EPCZ SERIES POST CAPS[1,2]

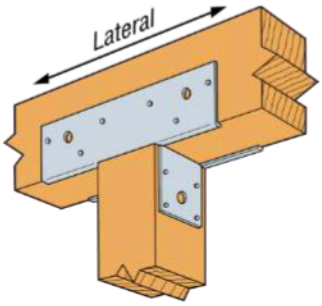
MODEL NO.	Min. Post Size (in)	POST CAP DIMENSIONS (in)					NAILS[8,9] (Quantity-Type)			ALLOWABLE LOADS[3,4,5] (lbs)			
										PC & PCZ		EPC & EPCZ	
		Width for Beam (W_1)	Width for Post (W_2)	Metal Flange Lengths			Into The Post	PC & PCZ Post Cap	EPC & EPCZ Post Cap	Uplift[6]	Lateral[7]	Uplift[6]	Lateral[7]
				PC/ EPC & PCZ/ EPCZ	PC & PCZ	EPC & EPCZ				C_D = 1.6	C_D = 1.6	C_D = 1.6	C_D = 1.6
				L_1	L_2	L_3							
PC44-16	4x4	3 9/16	3 9/16	2 5/8	11	7 3/8	8-16d	12-16d	8-16d	1,025	970	1,025	970
PC46-16	4x6		5 1/2	2 5/8	13	9 1/4	8-16d	12-16d	8-16d	1,025	970	1,025	970
PC48-16	4x8		7 1/2	2 5/8	15	11 1/4	8-16d	16-16d	12-16d	1,025	1,270	1,025	1,270
PC64-16	4x6	5 1/2	3 9/16	4 9/16	11	7 3/8	8-16d	12-16d	8-16d	1,025	970	1,025	970
PC66-16	6x6		5 1/2	4 9/16	13	9 1/4	8-16d	12-16d	12-16d	1,025	970	1,025	970
PC4Z	4x4	3 9/16	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,130	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,260	1,130	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,130	1,230
PC4RZ	4x4	4	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,130	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,260	1,130	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,130	1,230
PC6Z	4x4	5 1/2	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,435	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,295	1,435	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,435	1,230
PC6RZ	4x4	6	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,435	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,295	1,435	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,435	1,230
PC8Z	4x4	7 1/2	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,435	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,295	1,435	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,435	1,230
PC8RZ	4x4	8	-	2 5/8	7	5 1/4	8-10d	10-10d	10-10d	1,480	1,260	1,435	1,075
	4x6						8-10d	10-10d	10-10d	1,480	1,295	1,435	1,230
	4x8						8-10d	10-10d	10-10d	1,480	1,380	1,435	1,230

For **SI:** 1 inch = 25.4 mm, 1 lbs = .45 N.

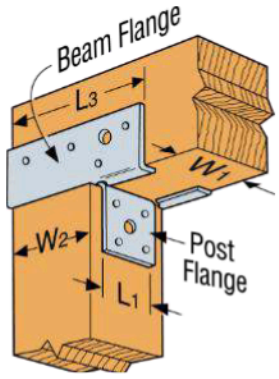
[1]The PCZ and EPCZ models shown in the table end with the letter Z, indicating that they have a G185 zinc coating in accordance with ASTM A653.
[2]The PCRZ and EPCRZ models shown in the table with the letter R, indicating that they are for rough cut sawn lumber.
[3]Allowable loads have been increased for wind or earthquake load with no further increase allowed; reduce where other loads govern.
[4]Post and beam may consist of multiple members provided members are connected independently of the post cap fasteners. The designer must determine the fasteners required to join members without splitting the wood.
[5]Spliced conditions must be detailed by the designer to transfer tension loads between spliced members by means other than the column cap.
[6]Allowable uplift loads for the PC and PCZ column caps do not apply to spliced beam conditions.
[7]Allowable lateral loads are parallel to the length of the supported wood beam, as shown in Figure 5.
[8]Applies to PCZ and EPCZ models only: 10d x 2 1/2 long nails may be used with no reduction for uplift and 0.85 of the table loads for lateral.
[9]Applies to PCZ and EPCZ models only: SD9 = 0.131" dia. X 1 ½" long screws may be used with no load reduction to table loads and PCZ uplift load of 1930 lbs.



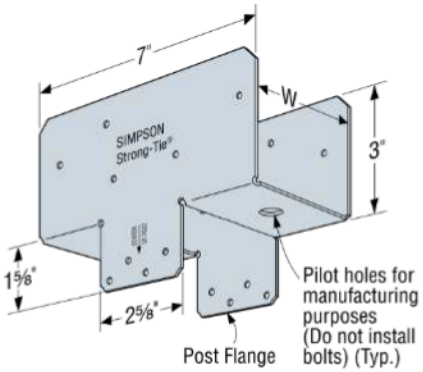
PC Post Cap



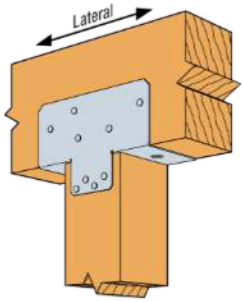
Typical PC Post Cap Installation



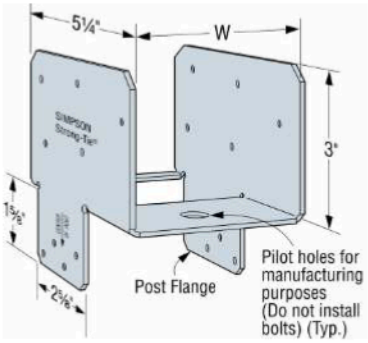
Typical EPC End Post Cap Installation



PCZ Post Cap



Typical PCZ Post Cap Installation



EPCZ End Post Cap

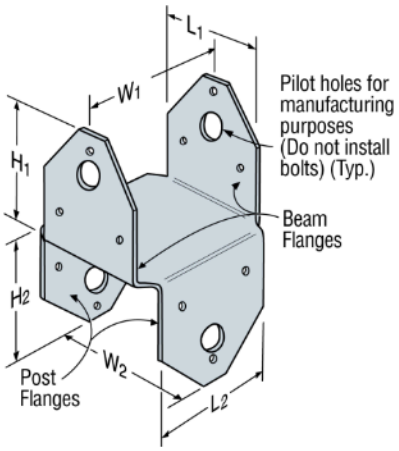
FIGURE 5—PC/EPC AND PCZ/EPCZ POST CAPS

TABLE 6—BC AND BCS POST CAPS[1]

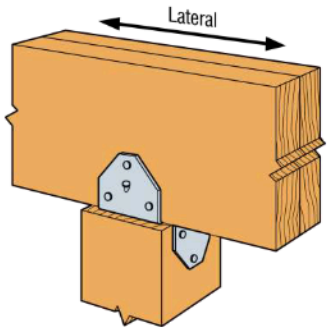
MODEL NO.	POST CAPS DIMENSIONS (in.)						NAILS[2] (Quantity-Type)		ALLOWABLE LOADS[3,4]	
	Width for Beam (W_1)	Width for Post (W_2)	Metal Flange Length		Metal Flange Height		Into the Wood Beam	Into the Wood Post	Uplift[5]	Lateral[6]
			Beam (L_1)	Post (L_2)	Beam (H_1)	Post (H_2)			C_D = 1.6	C_D = 1.6
BC4	$3^9/_{16}$	$3^9/_{16}$	$2^7/_8$	$2^7/_8$	3	3	6 –16d	6 –16d	605	1,000
BC46	$3^9/_{16}$	$5^1/_2$	$4^7/_8$	$4^7/_8$	$3^1/_2$	$2^1/_2$	12 –16d	6 –16d	945	1,000
BC4R	4	4	4	4	3	3	12 –16d	12 –16d	605	1,000
BC6	$5^1/_2$	$5^1/_2$	$4^3/_8$	$4^3/_8$	$3^3/_8$	$3^3/_8$	12 –16d	12 –16d	1,185	1,825
BC6R	6	6	6	6	3	3	12 –16d	12 –16d	1,185	1,825
BC8	$7^1/_2$	$7^1/_2$	$7^1/_2$	$7^1/_2$	4	4	12 –16d	12 –16d	1,660	1.825
BCS2-2/4	$3^1/_8$	$3^9/_{16}$	$2^7/_8$	$2^7/_8$	$2^{15}/_{16}$	$2^{15}/_{16}$	8 –10d	6 –10d	895	890
BCS2-3/6	$4^5/_8$	$5^9/_{16}$	$4^3/_8$	$2^7/_8$	$3^{15}/_{16}$	$2^{15}/_{16}$	12 –16d	6 –16d	895	1,330

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

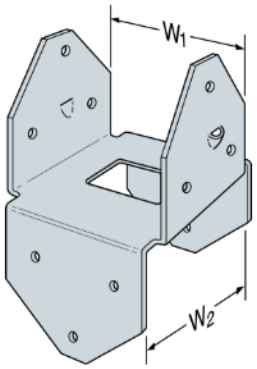
[1]The BCS2-2/4 post cap is designed for the connection of double 2x's to a nominally 4-inch-wide post, and the BCS2-3/6 post cap is designed for the connection of triple 2x's to a nominally 6-inch-wide post.
[2]The BCS has slant nail holes for nails that must be installed into the beam at a 45-degree angle and penetrate into the end grain of the supporting post. Nails must be minimum $3^1/_2$-inches long (i.e., 16d common nails).
[3]Tabulated allowable load capacities must be selected based on duration of load as permitted by the applicable building code.
[4]Spliced beams, where the ends of two beams are supported by the wood post and connected to the BC post cap connector, are not permitted. The supported wood beam must be continuous.
[5]Allowable uplift and lateral loads have been increased for wind or earthquake loading with no further increase allowed. The allowable loads must be reduced when other load durations govern.
[6]Allowable lateral loads are parallel to the length of the supported wood beam, as shown in Figure 6.



Typical BCS Post Cap Installation



BCS2-2/4 Post Cap U.S. Patent 5,603,580



Slant Nail Holes, typical both sides. (See footnote 2)

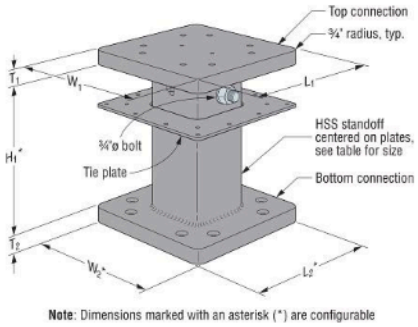
FIGURE 6—BC AND BCS POST CAPS

TABLE 7—MPSC DIMENSIONS

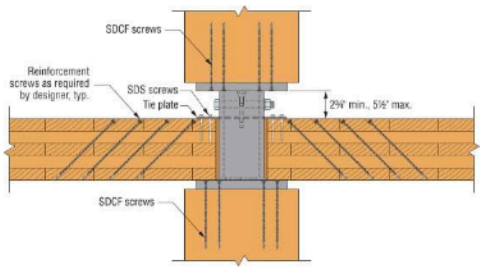
MODEL NO.	Top Plate Size (in.)			Bottom Plate Size (in.)			Height (in.)	Standoff Dimensions (Standard HSS Sizes)
	W_1	L_1	T_1	$W_{2,min}$[2]	$L_{2,min}$[2]	T_2	H_1	
MPSC100	$8^3/_4$	$8^3/_4$	$^3/_4$	$8^3/_4$	$8^3/_4$	$^3/_4$	Configurable Between 8" Min, 16" Max	$4x4x^5/_{16}$
MPSC100U	$8^3/_4$	$8^3/_4$	$^3/_4$	$8^3/_4$	$8^3/_4$	$^3/_4$		$4x4x^5/_{16}$
MPSC150	$9^1/_2$	$9^1/_2$	$^7/_8$	$9^1/_2$	$9^1/_2$	$^7/_8$		$5x5x^3/_8$
MPSC200	$10^1/_4$	$10^1/_4$	1	$10^1/_4$	$10^1/_4$	1		$5x5x^1/_2$
MPSC250	11	11	1	11	11	1		$6x6x^1/_2$
MPSC300	$11^3/_4$	$11^3/_4$	$1^1/_4$	$11^3/_4$	$11^3/_4$	$1^1/_4$		$6x6x^5/_8$

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

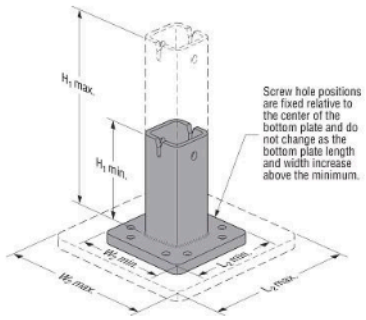
[1]Minimum clear distance from top of wood panel to bottom of connector top plate = $2\text{-}^3/_4$"
[2]The bottom plate width/length dimension W2, L2 are configurable. The minimum length/width dimensions are shown in the table. Maximum W_2, L_2 dimensions = 18". Location of fasteners holes remains the same relative to the HSS component.



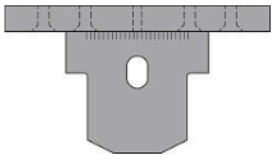
MPSC Column Connection



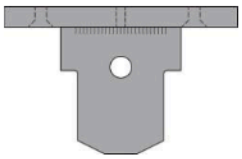
MPSC Typical Installation with CLT Panels and Glulam Columns



Configurability of MPSC Bottom Connection



MPSC200 Top Connection Front Elevation (Others Similar)
Knife Plate is $^3/_8$-inch-thick (9.53 mm)



MPSC100U Top Connection Front Elevation
Knife Plate is $^3/_8$-inch-thick (9.53 mm)

FIGURE 7—MPSC

TABLE 8—MPSC ALLOWABLE LOADS

MODEL NO.	Minimum Column Width/Depth[3] (in.)	Fasteners (Per Member)	Column Material[5]	Allowable Load (lb.)			
				Download[1,4]	Uplift[2]	Lateral[2]	
				(100/125)	(160)	(100)	(160)
MPSC100	$8^3/_4$	(4) SDCF10x200	DF/SP Glulam	100,500	-	1,915	2,785
			SCL CLT		-	1,850	2,785
MPSC100U	$8^3/_4$	(4) SDCF10x200	DF/SP Glulam	100,500	6,055	1,915	2,785
			SCL CLT		5,785	1,850	2,785
MPSC150	$9^1/_2$	(8) SDCF10x200	DF/SP Glulam	158,500	-	4,090	4,880
			SCL CLT		-	3,915	4,245
MPSC200	$10^1/_4$	(8) SDCF10x200	DF/SP Glulam	201,000	-	4,090	4,880
			SCL CLT		-	3,915	4,245
MPSC250	11	(12) SDCF10x200	DF/SP Glulam	256,000	-	6,510	6,545
			SCL CLT		-	6,045	6,045
MPSC300	$11^3/_4$	(12) SDCF10x200	DF/SP Glulam	300,000	-	6,510	6,545
			SCL CLT		-	6,045	6,045

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

[1]Download shall be reduced where limited by the capacity of the post.
[2]Uplift & Lateral loads have been increased for wind or seismic with no further increase allowed. Reduce where other loads govern.
[3]Column Width/Depth must be greater than or equal to Top Plate and Bottom Plate minimum width/length to achieve tabulated allowable download capacity.
[4]Allowable downloads are based on Fc = 2,200 psi for MPSC250 & MPSC300, and Fc = 1,950 psi for all others. For lower Fc, reduce allowable download proportionally.
[5]Uplift and lateral allowable loads for SCL CLT columns are based on a minimum equivalent specific gravity of 0.40 for fastener withdrawal capacity, and 0.49 for fastener lateral capacity.
[6]For simultaneous uplift and lateral loads, the connector must be evaluated using the Unity Equation:
Design Uplift/Allowable Uplift + Design Lateral/Allowable Lateral ≤ 1.0.

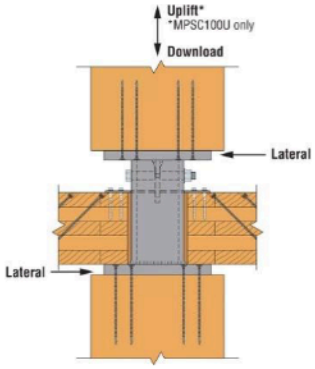


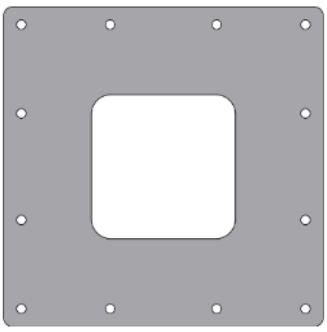
FIGURE 8—MPSC Loading Diagram

TABLE 9—MPSC TIE PLATE DIMENSIONS AND ALLOWABLE LOADS[2]

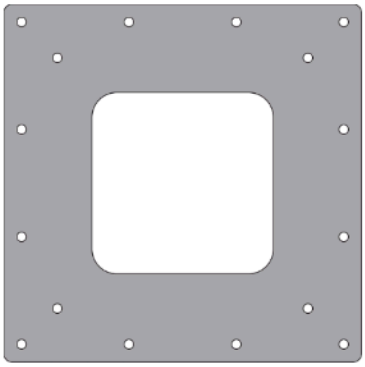
MODEL NO.	Applicable MPSC Model No.'s	Tie Plate Size	Fasteners		Allowable Load (lb.)[3]	
			Type	Quantity	Panel Material[1]	Lateral (100)
MPSC-TP4	MPSC100	9"x9"x7 ga.	SDS25300	12	SPF CLT	3,600
	MPSC100U				DF CLT/MPP	5,040
MPSC-TP5	MPSC150	10"x10"x7 ga.	SDS25300	16	SPF CLT	4,800
	MPSC200				DF CLT/MPP	6,720
MPSC-TP6	MPSC250	12"x12"x7 ga.	SDS25300	20	SPF CLT	6,000
	MPSC300				DF CLT/MPP	8,400

For **SI:** 1 inch = 25.4 mm, 1 lbs = 4.45 N.

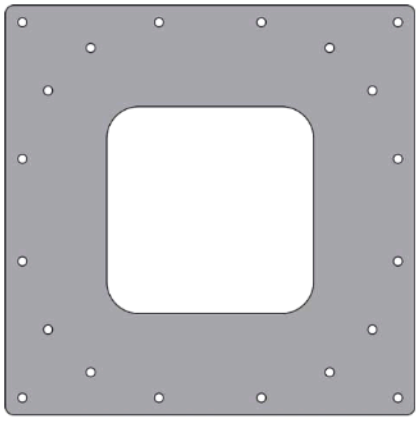
[1]Screw capacities are based upon SPF CLT (SG=0.42), and DF/MPP Grade F10 or better (SG≥0.50). Panel thickness shall be larger than 3 inches.
[2]Connection lateral capacity is the lesser of the allowable lateral load in Table 8 and the allowable lateral load in Table 9.
[3]Allowable loads must not be increased for duration of load.



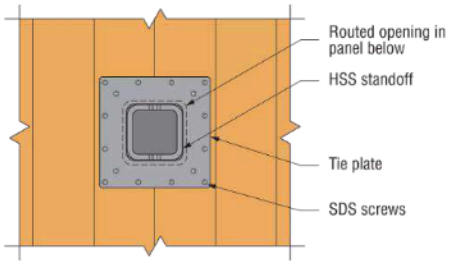
MPSC-TP4



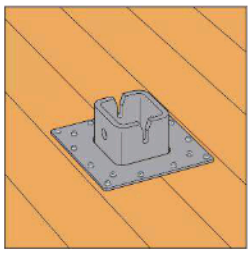
MPSC-TP5



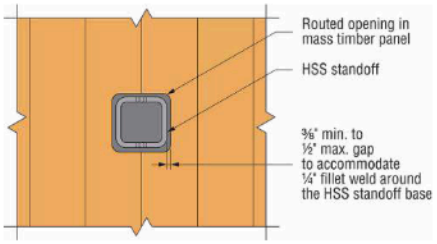
MPSC-TP6



MPSC Tie Plate Typical Installation with CLT Panels – Plan View



MPSC Tie Plate Typical Installation with CLT Panels – Perspective View



MPSC Tie Plate Routing Diagram

FIGURE 9—MPSC TIE PLATE

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
Section: 06 05 23.30—Connectors - General

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® COLUMN CAPS AND POST CAPS FOR WOOD CONSTRUCTION

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that Simpson Strong-Tie® column caps and post caps for wood construction, described in ICC-ES evaluation report ESR-2604, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2026 *City of Los Angeles Building Code* (LABC)
- 2026 *City of Los Angeles Residential Code* (LARC)

2.0 CONCLUSIONS

The Simpson Strong-Tie® column caps and post caps for wood construction, described in Sections 2.0 through 7.0 of the evaluation report ESR-2604, comply with the LABC Chapter 23, and the LARC, and are subject to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The Simpson Strong-Tie® column caps and post caps for wood construction, described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report ESR-2604.
- The design, installation, conditions of use and identification of Simpson Strong-Tie® column caps and post caps for wood construction are in accordance with the 2024 *International Building Code®* (IBC) and 2024 *International Residential Code®* (IRC) provisions, as applicable, noted in the evaluation report ESR-2604.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16, 17 and 23, as applicable.
- The connections are not approved to resist uplift forces from wood shear walls.
- Allowable loads must be reduced when load durations with lower value Cd (than what is in the tables) govern.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.

This supplement expires concurrently with the evaluation report, reissued January 2026 and revised July 30, 2026.

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ICC-ES Evaluation Report

ESR-2604 FL Supplement

Reissued January 2026

Revised July 30, 2026

This report remains under an active continuous compliance program through the end of January 2027.

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DIVISION: 06 00 00—WOOD, PLASTICS, AND COMPOSITES
Section: 06 05 23.30—Connectors - General

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® COLUMN CAPS AND POST CAPS FOR WOOD CONSTRUCTION

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the Simpson Strong-Tie® column caps and post caps for wood construction, described in ICC-ES evaluation report ESR-2604, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2023 *Florida Building Code—Building*
- 2023 *Florida Building Code—Residential*

2.0 CONCLUSIONS

The Simpson Strong-Tie® column caps and post caps for wood construction, described in Sections 2.0 through 7.0 of ICC-ES evaluation report ESR-2604, comply with the *Florida Building Code—Building*, and the *Florida Building Code—Residential*. The design requirements must be determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential,* as applicable. The installation requirements noted in ICC-ES evaluation report ESR-2604 for the 2021 *International Building Code®* meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential,* as applicable.

Use of the Simpson Strong-Tie® column caps and post caps for wood construction has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building*, and the *Florida Building Code—Residential* with the following condition:

a. For connections subject to uplift, the connection must be designed for no less than 700 pounds (3114 N).

For products falling under Florida Rule 61G20-3, verification that the report holder's quality assurance program is audited by a quality assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official when the report holder does not possess an approval by the Commission). Florida Rule 61G20-3 is applicable to products and/or systems which comprise the building envelope and structural frame for compliance with the structural requirements of the Florida Building Code.

This supplement expires concurrently with the evaluation report ESR-2604, reissued January 2026 and revised July 30, 2026.

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