Titen HD[®] Heavy-Duty Screw Anchor

A high-strength screw anchor for use in cracked and uncracked concrete, as well as uncracked masonry. The Titen HD offers low installation torgue and outstanding performance. Designed for use in dry, interior, non-corrosive environments or temporary outdoor applications.

Features

Mechanical Anchors

- Tested in accordance with ACI 355.2, AC193 and AC106
- · Qualified for static and seismic loading conditions
- Thread design undercuts to efficiently transfer the load to the base material
- Standard fractional sizes
- Specialized heat-treating process creates tip hardness for better cutting without compromising the ductility
- No special drill bit required designed to install using standard-sized ANSI tolerance drill bits
- Hex-washer head requires no separate washer, unless required by code, and provides a clean installed appearance
- Removable ideal for temporary anchoring (e.g. formwork, bracing) or applications where fixtures may need to be moved
- · Reuse of the anchor will not achieve listed loads and is not recommended

Codes: ICC-ES ESR-2713 (concrete);

ICC-ES ESR-1056 (masonry); City of LA Supplement within ESR-2713 (concrete); City of LA Supplement within ESR-1056 (masonry); Florida FL15730 (concrete and masonry); FM 3017082, 3035761 and 3043442; Multiple DOT listings

Material: Carbon steel

Coating: Zinc plated or mechanically galvanized.

Not recommended for permanent exterior use or highly corrosive environments.

Installation

- Holes in steel fixtures to be mounted should match the diameter specified in the table below.
 - Use a Titen HD screw anchor one time only installing the anchor multiple times may result in excessive thread wear and reduce load capacity.
- Do not use impact wrenches to install into hollow CMU.
- Caution: Oversized holes in base material will reduce or eliminate the A mechanical interlock of the threads with the base material and reduce the anchor's load capacity.
- 1. Drill a hole in the base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed. Drill the hole to the specified embedment depth plus minimum hole depth overdrill (see table below) to allow the thread tapping dust to settle, and blow it clean using compressed air. (Overhead installations need not be blown clean.) Alternatively, drill the hole deep enough to accommodate embedment depth and the dust from drilling and tapping.
- 2. Insert the anchor through the fixture and into the hole.
- 3. Tighten the anchor into the base material until the hex-washer head contacts the fixture.

Additional Installation Information

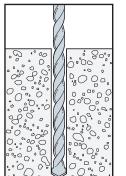
Titen HD [®] Diameter (in.)	Wrench Size (in.)	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
1⁄4	3⁄8	3% to 7⁄16	1⁄8
3⁄8	9⁄16	½ to %16	1⁄4
1/2	3⁄4	5% to 11/16	1⁄2
5⁄8	15/16	3⁄4 t0 13⁄16	1⁄2
3⁄4	11/8	7% to 15/16	1/2

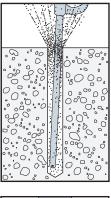
Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or thinner cold-formed steel members.



Titen HD **Screw Anchor**

Installation Sequence





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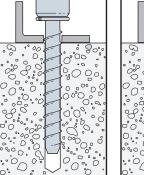
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Serrated teeth on the

Cracked

Concrete



Titen HD® Heavy-Duty Screw Anchor

Countersunk Head Style

The countersunk head style is for applications that require a flush-mount profile. Countersinking also leaves a cleaner surface appearance for exposed through-set applications. The anchor head's 6-lobe drive eases installation and is less prone to stripping than traditional recessed anchor heads.

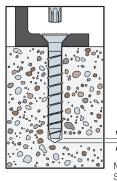
Features

- Available in many standard lengths in 1/4" and 3/8" diameters
- Driver bit included in each box

Codes: ICC-ES ESR-2713 (concrete); ICC-ES ESR-1056 (masonry); City of LA Supplement within ESR-2713 (concrete); City of LA Supplement within ESR-1056 (masonry); Florida FL15730 (concrete and masonry)

Material: Carbon steel

Coating: Zinc plated



Additional Installation Information

Titen HD Diameter (in.)	Bit Size	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
1⁄4	T30	3% to 7⁄16	1⁄8
3⁄8	T50	1⁄2 t0 %16	1⁄4

Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or thinner cold-formed steel members.

Minimum overdrill See table



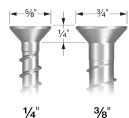


Cracked

Concrete



6-lobe drive



Titen HD Countersunk **Head Style**

Washer-Head Head Style

The washer-head design is commonly used where a minimal head profile is necessary. The model is offered in sizes suitable for use in sill plate applications, and the washer head's low installed profile means modular wall and floor systems can be installed on top with no need for notching the wall framing to accommodate the anchor. The anchor's 6-lobe drive eases driving and is less prone to stripping.

Features

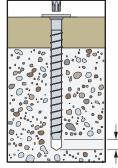
- Available in many standard lengths in ½" and %" diameters
- Driver bit included in each box

Codes: ICC-ES ESR-2713 (concrete);

City of LA Supplement within ESR-2713 (concrete) Florida FL15730 (concrete)

Material: Carbon steel

Coating: Zinc plated



Additional Installation Information

Titen HD Diameter (in.)	Bit Size	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
1⁄2	T50	5% to 11/16	1/2
5⁄8	T60	3⁄4 t0 ¹³ ⁄16	1/2

Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or thinner cold-formed steel members.



Minimum overdrill. See table

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Titen HD® Heavy-Duty Screw Anchor

Titen HD Anchor Product Data — Zinc Plated

Size	Model	Thread	Drill Bit	Wrench	Qua	ntity
(in.)	No.	Length (in.)	Diameter (in.)	Size (in.)	Вох	Carton
1⁄4 x 1 7⁄8	THDB25178H	1 1/2	1/4	3⁄8	100	500
1⁄4 x 23⁄4	THDB25234H	23⁄8	1/4	3⁄8	50	250
1⁄4 x 3	THDB25300H	2%	1/4	3⁄8	50	250
1⁄4 x 31⁄2	THDB25312H	31/8	1/4	3%8	50	250
1⁄4 x 4	THDB25400H	3%	1/4	3⁄8	50	250
3∕8 x 13⁄4	THD37134H [†]	1 1⁄4	3⁄8	9⁄16	50	250
3∕8 X 21⁄2	THD37212H [†]	2	3⁄8	9⁄16	50	200
3∕8 x 3	THD37300H	21⁄2	3⁄8	9⁄16	50	200
3∕8 x 4	THD37400H	31⁄2	3⁄8	9⁄16	50	200
³∕% x 5	THD37500H	41⁄2	3⁄8	9⁄16	50	100
3∕8 X 6	THD37600H	5½	3⁄8	9⁄16	50	100
1⁄2 x 3	THD50300H	21⁄2	1/2	3⁄4	25	100
½ x 4	THD50400H	31⁄2	1/2	3⁄4	20	80
½ x 5	THD50500H	41⁄2	1/2	3⁄4	20	80
½ x 6	THD50600H	5½	1/2	3⁄4	20	80
1⁄2 x 61⁄2	THD50612H	5½	1/2	3⁄4	20	40
1⁄2 x 8	THD50800H	5½	1/2	3⁄4	20	40
½ x 12	THD501200H	51⁄2	1/2	3⁄4	5	25
½ x 13	THD501300H	51⁄2	1/2	3⁄4	5	25
½ x 14	THD501400H	51⁄2	1/2	3⁄4	5	25
½ x 15	THD501500H	51⁄2	1/2	3⁄4	5	25
5∕8 x 4	THDB62400H	31⁄2	5⁄8	15/16	10	40
5∕8 x 5	THDB62500H	41⁄2	5⁄8	15/16	10	40
5∕8 X 6	THDB62600H	5½	5/8	15/16	10	40
5% x 6½	THDB62612H	5½	5⁄8	15/16	10	40
5% x 8	THDB62800H	5½	5/8	15/16	10	20
5∕8 x 10	THDB62100H	5½	5/8	15/16	10	20
3⁄4 x 4	THD75400H	31/2	3⁄4	1 1/8	10	40
3⁄4 x 5	THD75500H	41⁄2	3⁄4	1 1/8	5	20
3⁄4 x 6	THDT75600H	41⁄2	3⁄4	1 1/8	5	20
3⁄4 x 7	THD75700H	5½	3⁄4	1 1/8	5	10
3⁄4 x 81∕2	THD75812H	5½	3⁄4	3⁄4 11⁄8 5		10
3⁄4 x 10	THD75100H	5½	3⁄4	1 1/8	5	10

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† These models do not meet minimum embedment depth requirements for strength design and require maximum installation torque of 25 ft. - lb.

using a torque wrench, driver drill or cordless ¼" impact driver with a maximum permitted torque rating of 100 ft. – lb. 1. Length of anchor is measured from underside of head to end of anchor.

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Titen HD Anchor Product Data - Countersunk - Zinc Plated

Titen HD® Heavy-Duty Screw Anchor

	Size	Model	Thread	Drill Bit Diameter	Wrench Size	Qua	ntity
	(in.)	No.	Length (in.)	(in.)	(in.)	Вох	Carton
@	1⁄4 x 1 7⁄8	THDB25178CS	1 1⁄2	1⁄4	T30	100	500
@	1⁄4 x 23⁄4	THDB25234CS	23⁄8	1⁄4	T30	50	250
@	1⁄4 x 31⁄2	THDB25312CS	31⁄8	1⁄4	T30	50	250
@	1⁄4 x 41⁄2	THDB25412CS	41⁄8	1⁄4	T30	50	250
1	3∕8 x 21⁄2	THD37212CS ⁺	2	3⁄8	T50	50	200
1	3∕8 x 3	THD37300CS	21⁄2	3⁄8	T50	50	200
ø	3∕8 x 4	THD37400CS	31⁄2	3⁄8	T50	50	200
ø	3∕8 x 5	THD37500CS	41⁄2	3⁄8	T50	50	100

[†] This model does not meet minimum embedment depth requirements for strength design and require maximum installation torque of 25 ft. – lb. using a torque wrench, driver drill or cordless ¼" impact driver with a maximum permitted torque rating of 100 ft. – lb.

1. Length of anchor is measured from top of head to bottom of anchor.

Titen HD Anchor Product Data - Washer Head - Zinc Plated

	Size	Model	Thread	Drill Bit Diameter	Bit	Qua	ntity
	(in.)	No.	Length (in.)	(in.)	Size	Box	Carton
1	1⁄2 x 6	THD50600WH	5½	1/2	T50	15	60
1	1⁄2 x 8	THD50800WH	5½	1/2	T50	15	30
ø	5% x 6	THDB62600WH	51⁄2	5⁄8	T60	10	40
ø	5% x 8	THDB62800WH	5½	5⁄8	T60	10	20
ø	5% x 10	THDB62100WH	5½	5⁄8	T60	10	20

1. Length of anchor is measured from top of head to bottom of anchor.

Titen HD® Heavy-Duty Screw Anchor

Titen HD Anchor Product Data — Mechanically Galvanized

Size	Model	Thread	Drill Bit	Diameter Size		ntity
(in.)	No.	Length (in.)	(in.)	(in.)	Вох	Carton
3⁄8 X 3	THD37300HMG	21⁄2			50	200
3∕8 X 4	THD37400HMG	31⁄2	3%8	9/	50	200
3∕8 X 5	THD37500HMG	41⁄2	78	9⁄16	50	100
3∕8 X 6	THD37600HMG	51⁄2			50	100
1⁄2 X 4	THD50400HMG	31⁄2			20	80
1⁄2 x 5	THD50500HMG	41⁄2			20	80
1⁄2 X 6	THD50600HMG	51⁄2	1/2	3/4	20	80
½ x 6½	THD50612HMG	51⁄2	/2	94	20	40
1⁄2 X 8	THD50800HMG	51⁄2			20	40
½ x 12	THD501200HMG	51⁄2			5	20
5∕8 X 5	THDB62500HMG	41⁄2			10	40
5∕8 X 6	THDB62600HMG	51⁄2		¹⁵ ⁄16	10	40
5% x 6½	THDB62612HMG	51⁄2	5⁄8	'916	10	40
5% X 8	THDB62800HMG	51⁄2			10	20
³⁄4 x 5	THD75500HMG	41⁄2			5	20
3⁄4 X 6	THDT75600HMG	41⁄2	3⁄4	11/	5	20
3⁄4 x 81⁄2	THD75812HMG	51⁄2	3⁄4 11⁄8	1 1/8	5	10
3⁄4 x 10	THD75100HMG	5½			5	10

Mechanical galvanizing meets ASTM B695, Class 65, Type 1. Intended for some pressure-treated wood sill plate applications. Not for use in other corrosive or outdoor environments. See p. 261 or visit **strongtie.com/info** for more corrosion information.

Titen HD Installation Information and Additional Data¹

Ohovostavistis	Cumhal	Unite				Nor	ninal And	hor Dian	neter, d _a	(in.)			
Characteristic	Symbol	Units	1,	1⁄4		3⁄8		1⁄2		/8	3⁄4		
			Installa	tion Info	rmation								
Drill Bit Diameter	d _{bit}	in.	1,	/4	3,	/8	1,	/2	5	/8		3⁄4	
Baseplate Clearance Hole Diameter	d _c	in.	3,	/8	1,	2	5	/8	3,	/4		7⁄8	
Maximum Installation Torque	T _{inst,max}	ftlbf	2	4 ²	50) ²	6	5 ²	10	00 ²		150 ²	
Maximum Impact Wrench Torque Rating	T _{impact, max}	ftlbf	12	25 ³	15	0 ³	34	03	34	10 ³		385 ³	
Minimum Hole Depth	h _{hole}	in.	1 3⁄4	2%	23⁄4	3½	3¾	41⁄2	41⁄2	6	41⁄2	6	6¾
Nominal Embedment Depth	h _{nom}	in.	1 5%	21⁄2	21⁄2	31⁄4	31⁄4	4	4	5½	4	5½	6¼
Critical Edge Distance	C _{ac}	in.	3	6	211/16	3%	3%16	4½	41⁄2	6%	6	6%	75⁄16
Minimum Edge Distance	C _{min}	in.	1	1/2					13⁄4				
Minimum Spacing	S _{min}	in.	1	1/2				3			2¾		3
Minimum Concrete Thickness	h _{min}	in.	31⁄4	31⁄2	4	5	5	6¼	6	81⁄2	6	8¾	10
			Ado	ditional E	Data								
Anchor Category	Category	_						1					
Yield Strength	f _{ya}	psi	100	,000,					97,000				
Tensile Strength	f _{uta}	psi	125	,000,					110,000				
Minimum Tensile and Shear Stress Area	A _{se}	in ²	0.0)42	0.0	99	0.1	83	0.2	276		0.414	
Axial Stiffness in Service Load Range — Uncracked Concrete	β_{uncr}	lb./in.	. 202,000 672,000										
Axial Stiffness in Service Load Range — Cracked Concrete	β_{cr}	lb./in.	173	,000					345,000				

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17 and ACI 318-11 Appendix D.

2. T_{inst.max} is the maximum permitted installation torque for the embedment depth range covered by this table using a torque wrench.

3. T_{impact,max} is the maximum permitted torque rating for impact wrenches for the embedment depth range covered by this table.

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Titen HD Tension Strength Design Data¹

Characteristic	Sumbol	Units				Norr	ninal And	hor Diar	neter, d _a	(in.)			
Gharacteristic	Symbol	Units	1	/4	3⁄8 1		1⁄2		5⁄8		3⁄4		
Nominal Embedment Depth	h _{nom}	in.	1%	21⁄2	21⁄2	31⁄4	31⁄4	4	4	5½	4	5½	6¼
Steel Strength in	Tension -	— ACI 3	318-14 \$	Section 1	7.4.1 or	ACI 318	-11 Sect	ion D.5. ⁻	1				
Tension Resistance of Steel	N _{sa}	lb.	5,1	95	10,	890	20,	130	30,3	360		45,540	
Strength Reduction Factor — Steel Failure	ϕ_{sa}	—						0.65 ²					
Concrete Breakout Stre	ngth in Te	nsion ⁶ -	— ACI 3	18-14 S	ection 17	7.4.2 or A	ACI 318-	11 Secti	on D.5.2				
Effective Embedment Depth	h _{ef}	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Critical Edge Distance ⁶	Cac	in.	3	6	211/16	3%	3%16	41⁄2	4 1⁄2	6%	6	6%	75⁄16
Effectiveness Factor — Uncracked Concrete	k _{uncr}		30				24				27	2	4
Effectiveness Factor — Cracked Concrete	k _{cr}	—						17					
Modification Factor	$\Psi_{c,N}$							1.0					
Strength Reduction Factor — Concrete Breakout Failure	ϕ_{cb}	—						0.657					
Pullout Strength i	n Tension	— ACI	318-14	Section	17.4.3 o	r ACI 318	3-11 Sec	tion D.5	.3				
Pullout Resistance, Uncracked Concrete (f' $_{\rm c}$ = 2,500 psi)	N _{p,uncr}	lb.	3	3	2,7004	3	<u>3</u>	3	3	9,810 ⁴	<u>3</u>	3	3
Pullout Resistance, Cracked Concrete ($f'_c = 2,500$ psi)	N _{p,cr}	lb.	3	1,9054	1,2354	2,7004	3	3	3,0404	5,570 ⁴	<u>3</u>	6,070 ⁴	7,195 ⁴
Strength Reduction Factor — Concrete Pullout Failure	ϕ_{ρ}	_						0.655					
Tension Strength for Seismi	c Applicat	tions —	- ACI 31	3-14 Sec	tion 17.4	4.2.3.3 o	r ACI 31	8-11 Sec	ction D.3.	3.3			
Nominal Pullout Strength for Seismic Loads ($f'_c = 2,500 \text{ psi}$)	N _{p,eq}	lb.	3	1,9054	1,2354	2,7004	3	3	3,0404	5,570 ⁴	3,8404	6,070 ⁴	7,195 ⁴
Strength Reduction Factor — Breakout or Pullout Failure	ϕ_{eq}	_						0.655					

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.

2. The tabulated value of ϕ_{sa} applies when the load combinations of Section 1605.2.1 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ_{sa} must be determined in accordance with ACI 318-11 D.4.4. Anchors are considered brittle steel elements.

3. Pullout strength is not reported since concrete breakout controls.

4. Adjust the characteristic pullout resistance for other concrete compressive strengths by multiplying the tabular value by (fc. specified / 2,500)^{0.5}.

5. The tabulated value of ϕ_p or ϕ_{eq} applies when the load combinations of Section 1605.2.1 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used and the requirements of ACI 318-14 17.3.3.(c) or ACI 318-11 D.4.3(c) for Condition B are met. If the load combinations of ACI 318-11 Appendix C are used, appropriate value of ϕ must be determined in accordance with ACI 318-11 Section D.4.4(c).

6. The modification factor $\Psi_{cp,N} = 1.0$ for cracked concrete. Otherwise, the modification factor for uncracked concrete without supplementary reinforcement to control splitting is either:

)
$$\Psi_{cp,N} = 1.0$$
 if $c_{a,min} \ge c_{ac}$ or (2) $\Psi_{cp,N} = \frac{c_{a,min}}{c_{ac}} \ge \frac{1.5h_{ef}}{c_{ac}}$ if $c_{a,min} < c_{ac}$

The modification factor, $\Psi_{cp,N}$ is applied to the nominal concrete breakout strength, N_{cb} or N_{cbg} .

7. The tabulated value of ϕ_{cb} applies when both the load combinations of Section 1605.2.1 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used and the requirements of ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition B are met. Condition B applies where supplementary reinforcement is not provided. For installations where complying supplementary reinforcement can be verified, the ϕ_{cb} factors described in ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition A are allowed. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ_{cb} must be determined in accordance with ACI 318-11 D.4.4(c).

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Titen HD Shear Strength Design Data¹

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Characteristic	Symbol	Unit				Nor	ninal And	hor Dian	neter, d _a	(in.)			
onaracteristic	Gymbol	onne	1⁄4		3⁄8		1⁄2		5⁄8		3⁄4		
Nominal Embedment Depth	h _{nom}	in.	1%	21⁄2	21⁄2	31⁄4	31⁄4	4	4	5½	4	5½	6¼
			Steel Str	ength in	Shear								
Shear Resistance of Steel	V _{sa}	lb.	2,0)20	4,4	60	7,4	55	10,	000	14,950	16,	840
Strength Reduction Factor — Steel Failure	ϕ_{sa}	—						0.60 ²					
		Concre	te Break	out Stren	igth in Sl	near							
Outside Diameter	da	in.	0.	25	0.3	375	0.5	600	0.6	625	0.750		
Load Bearing Length of Anchor in Shear	le	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Strength Reduction Factor — Concrete Breakout Failure	ϕ_{cb}	_						0.70 ³					
		Concr	ete Pryo	ut Streng	th in She	ear							
Coefficient for Pryout Strength	k _{cp}	lb.			1.0					2	.0		
Strength Reduction Factor — Concrete Pryout Failure	ϕ_{cp}	—	- 0.70 ⁴										
	Steel	Streng	rength in Shear for Seismic Applications										
Shear Resistance for Seismic Loads	V _{eq}	lb.	. 1,695 2,855 4,790 8,000 9,350										
Strength Reduction Factor — Steel Failure	ϕ_{eq}	_						0.60 ²					

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.

2. The tabulated value of \$\phi_{sa}\$ and \$\phi_{eq}\$ applies when the load combinations of Section 1605.2.1 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of \$\phi_{sa}\$ and \$\phi_{eq}\$ must be determined in accordance with ACI 318 D.4.4.

3. The tabulated value of \u03c6_{cb} applies when both the load combinations of Section 1605.2.1 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2 are used and the requirements of ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition B are met. Condition B applies where supplementary reinforcement is not provided. For installations where complying supplementary reinforcement can be verified, the \u03c6_{cb} factors described in ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition A are allowed. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of \u03c6_{cb} must be determined in accordance with ACI 318-11 D.4.4(c).

4. The tabulated value of φ_{cp} applies when both the load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 Section 9.2 are used and the requirements of ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) for Condition B are met. If the load combinations of ACI 318-11 Appendix C are used, appropriate value of φ_{cp} must be determined in accordance with ACI 318-11 Section D.4.4(c).

Titen HD Tension and Shear Strength Design Data for the Soffit of Normal-Weight or Sand-Lightweight Concrete over Steel Deck^{1,6,7}

						Nomina	I Anchor	Diameter	; d _a (in.)			
Characteristic	Symbol	Units			Lowe	^r Flute				Upper Flute		
Gharactensuc	Symbol	Units	Figu	Figure 2 Figure 1						Figure 2		ure 1
			1	/4	3,	/8	1,	/2	1,	4	3⁄8	1⁄2
Nominal Embedment Depth	h _{nom}	in.	1%	21⁄2	1 7⁄8	21⁄2	2	31⁄2	1 5⁄8	21⁄2	1 7⁄8	2
Effective Embedment Depth	h _{ef}	in.	1.19	1.94	1.23	1.77	1.29	2.56	1.19	1.94	1.23	1.29
Pullout Resistance, concrete on steel deck (cracked) ^{2,3,4}	N _{p,deck,cr}	lb.	420	535	375	870	905	2,040	655	1,195	500	1,700
Pullout Resistance, concrete on steel deck (uncracked) ^{2,3,4}	N _{p,deck,uncr}	lb.	995	1,275	825	1,905	1,295	2,910	1,555	2,850	1,095	2,430
Steel Strength in Shear, concrete on steel deck5	V _{sa, deck}	lb.	1,335	1,745	2,240	2,395	2,435	4,430	2,010	2,420	4,180	7,145
Steel Strength in Shear, Seismic	V _{sa, deck,eq}	lb.	870	1,135	1,434	1,533	1,565	2,846	1,305	1,575	2,676	4,591

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.

 Concrete compressive strength shall be 3,000 psi minimum. The characteristic pullout resistance for greater compressive strengths shall be increased by multiplying the tabular value by (fⁱ_{c,specified} /3,000)^{0.5}.

3. For anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies,

as shown in Figure 1 and Figure 2, calculation of the concrete breakout strength may be omitted.

4. In accordance with ACI 318-14 Section 17.4.3.2 or ACI 318-11 Section D.5.3.2, the nominal pullout strength in cracked concrete for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies N_{p,deck,cr} shall be substituted for N_{p,cr}. Where analysis indicates no cracking at service loads, the normal pullout strength in uncracked concrete N_{p,deck,uncr} shall be substituted for N_{p,uncr}.

5. In accordance with ACI 318-14 Section 17.5.1.2(C) or ACI 318-11 Section D.6.1.2(c), the shear strength for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies $V_{sa.deck.eg}$ and $V_{sa.deck.eg}$ shall be substituted for V_{sa} .

6. Minimum edge distance to edge of panel is 2hef.

7. The minimum anchor spacing along the flute must be the greater of 3h_{eft} or 1.5 times the flute width.

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Titen HD Anchor Tension and Shear Strength Design Data in the Topside of Normal-Weight Concrete or Sand-Lightweight Concrete over Steel Deck

Bana Eightwoight Bohon				
			Nominal Anchor	Diameter, d _a (in.)
Design Information	Symbol	Units	Figure 3	Figure 3
			1⁄4	3⁄8
Nominal Embedment Depth	h _{nom}	in.	1 %	21⁄2
Effective Embedment Depth	h _{ef}	in.	1.19	1.77
Minimum Concrete Thickness	h _{min,deck}	in.	21⁄2	31⁄4
Critical Edge Distance	C _{ac,deck,top}	in.	3¾	7 1⁄4
Minimum Edge Distance	C _{min,deck,top}	in.	31⁄2	3
Minimum Spacing	S _{min,deck,top}	in.	31⁄2	3

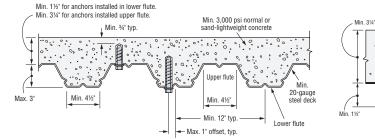
 For anchors installed in the topside of concrete-filled deck assemblies, as shown in Figures 2 and 3, the nominal concrete breakout strength of a single anchor or group of anchors in shear, V_{cb} or V_{cbg}, respectively, must be calculated in accordance with ACI 318-14 Section 17.5.2 or ACI 318-11 Section D.6.2, using the actual member thickness, h_{min,deck}, in the determination of A_{vc}.

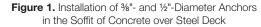
2. Design capacity shall be based on calculations according to values in the tables featured on p. 84.

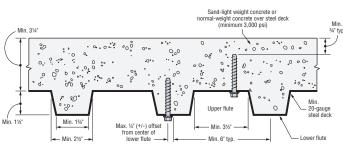
3. Minimum flute depth (distance from top of flute to bottom of flute) is 11/2" (see Figures 2 and 3).

4. Steel deck thickness shall be minimum 20 gauge.

5. Minimum concrete thickness (h_{min,deck}) refers to concrete thickness above upper flute (see Figures 2 and 3).







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Figure 2. Installation of 1/4"-Diameter Anchors in the Soffit of Concrete over Steel Deck

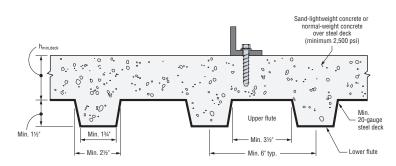


Figure 3. Installation of ¼"- and %"-Diameter Anchors in the Topside of Concrete over Steel Deck

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Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU

0:	D-:'II D'4	Minimum		Critical Edge		Critical	Values for 8" Lightweight, Medium-Weight or Normal-Weight Grout-Filled CMU				
Size in. (mm)	Drill Bit Diameter in.	Embedment Depth in.	Distance C _{crit}	Distance C _{min}	Spacing Distance in. (mm)	Tension Load		Shear Load			
(1111)		(mm)	in. (mm)	in. (mm)		Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable Ib. (kN)		
	Anchor Installed in the Face of the CMU Wall (See Figure 4)										
1/4	1⁄4	2½	4	1 ¼	4	2,050	410	2,500	500		
(6.4)		(64)	(102)	(32)	(102)	(9.1)	(1.8)	(11.1)	(2.2)		
3%	3⁄8	2¾	12	4	6	2,390	480	4,340	870		
(9.5)		(70)	(305)	(102)	(152)	(10.6)	(2.1)	(19.3)	(3.9)		
1⁄2	1⁄2	31⁄2	12	4	8	3,440	690	6,920	1,385		
(12.7)		(89)	(305)	(102)	(203)	(15.3)	(3.1)	(30.8)	(6.2)		
5%	5⁄8	41⁄2	12	4	10	5,300	1,060	10,420	2,085		
(15.9)		(114)	(305)	(102)	(254)	(23.6)	(4.7)	(46.4)	(9.3)		
3⁄4	3⁄4	5½	12	4	12	7,990	1,600	15,000	3,000		
(19.1)		(140)	(305)	(102)	(305)	(35.5)	(7.1)	(66.7)	(13.3)		

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.

3. The masonry units must be fully grouted.

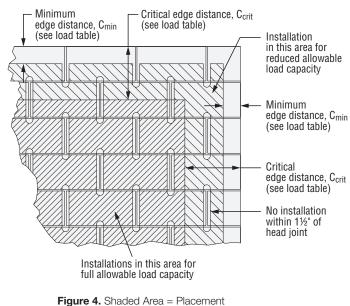
Mechanical Anchors

4. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.

5. Embedment depth is measured from the outside face of the concrete masonry unit.

6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.

7. Refer to allowable load-adjustment factors for spacing and edge distance on pp. 90-91.



for Full and Reduced Allowable Load Capacity in Grout-Filled CMU SIMPSON

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Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU

	D.:'!! D'#	Embedment	Minimum	8" Hollow CMU Loads Based on CMU Strength							
Size in. (mm)	Drill Bit Diameter in.	Depth⁴ in.	Edge Distance in.	Tensio	n Load	Shea	r Load				
(11111)		(mm)	(mm)	Ultimate Ib. (kN)	Allowable Ib. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)				
	Anchor Installed in Face Shell (See Figure 5)										
3%8 (9.5)	3⁄8	1¾ (45)	4 (102)	720 (3.2)	145 (0.6)	1,240 (5.5)	250 (1.1)				
½ (12.7)	1/2	1¾ (45)	4 (102)	760 (3.4)	150 (0.7)	1,240 (5.5)	250 (1.1)				
⁵% (15.9)	5⁄8	1¾ (45)	4 (102)	800 (3.6)	160 (0.7)	1,240 (5.5)	250 (1.1)				
3⁄4 (19.1)	3⁄4	1¾ (45)	4 (102)	880 (3.9)	175 (0.8)	1,240 (5.5)	250 (1.1)				

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

Note: No installation within 4%" of bed joint of hollow masonry block wall.

2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.

3. The minimum specified compressive strength of masonry, f'_{m} , at 28 days is 1,500 psi.

4. Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional $\frac{1}{2}$ "- through 1 $\frac{1}{4}$ "-thick face shell.

5. Allowable loads may not be increased for short-term loading due to wind or seismic forces.

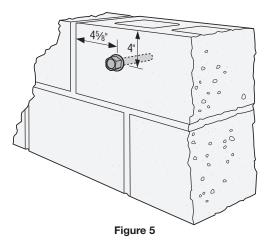
CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.

6. Do not use impact wrenches to install in hollow CMU.

7. Set drill to rotation-only mode when drilling into hollow CMU.

8. The tabulated allowable loads are based on one anchor installed in a single cell.

9. Distance from centerline of anchor to head joint shall be a minimum of 45%".



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Titen HD[®] Allowable Tension and Shear Loads in

8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall

Size Drill Bit in. Diamete (mm) in.		EMDed.	Minimum	Minimum	Critical Spacing Distance in. (mm)	8" Grout-Filled CMU Allowable Loads Based on CMU Strength, $f'_m = 1,500$ psi						
	Drill Bit Diameter		Edge Distance	End Distance		Tension		Shear Perpendicular to Edge		Shear Parallel to Edge		
	in.		in. (mm)	in. (mm)		Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	
	Anchor Installed in Cell Opening or Web (Top of Wall) (See Figure 6)											
1⁄2 (12.7)	1⁄2	4½ (114)	1¾ (45)	8 (203)	8 (203)	2,860 (12.7)	570 (2.5)	800 (3.6)	160 (0.7)	2,920 (13.0)	585 (2.6)	
5⁄8 (15.9)	5⁄8	4½ (114)	1¾ (45)	10 (254)	10 (254)	2,860 (12.7)	570 (2.5)	800 (3.6)	160 (0.7)	3,380 (15.0)	675 (3.0)	

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

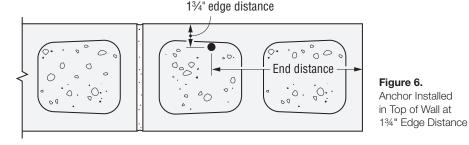
2. Values are for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.

3. The masonry units must be fully grouted.

4. The minimum specified compressive strength of masonry, f'_{m} , at 28 days is 1,500 psi.

5. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.

6. Loads are based on anchor installed in either the web or grout-filled cell opening in the top of wall.



			sion and Normal-V			d CMU Stei	mwall					
		Embod	Minimum	Minimum	Critical Spacing Distance in. (mm)	8" Grou	ut-Filled CMU AI	Based on CMU St	trength, $f'_m = 2,$	000 psi		
Size in.	Diameter in	Depth	Edge Distance in. (mm)	End Distance			Tension		Shear Perpendicular to Edge		Shear Parallel to Edge	
(mm) in.				in. (mm)		Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	
				Anch	or Installed i	in Cell Opening	(Top of Wall) (Se	e Figure 7)				
½ (12.7)	1⁄2	41⁄2	3	12	12	5,800	1,160	2,750	550	7,500	1,500	
5⁄8 (15.9)	5⁄8	(114)		(76)	(305)	(305)	(25.8)	(5.2)	(12.2)	(2.5)	(33.4)	(6.7)

1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.

2. Values are for 8"-wide, medium-weight and normal-weight concrete masonry units.

3. The masonry units must be fully grouted.

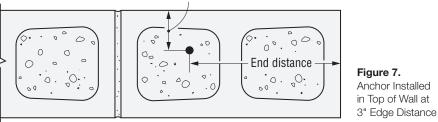
4. The minimum specified compressive strength of masonry, f'_m , at 28 days is 2,000 psi.

5. Allowable loads are not permitted to be increased for short-term loading due to wind or seismic forces.

6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.

7. Loads are based on anchor installed in grout-filled cell opening in the top of wall.

3" edge distance



* See p. 12 for an explanation of the load table icons.

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Titen HD Allowable Tension Loads for 8" Lightweight, Medium-Weight and Normal-Weight CMU Chair Blocks Filled with Normal-Weight Concrete

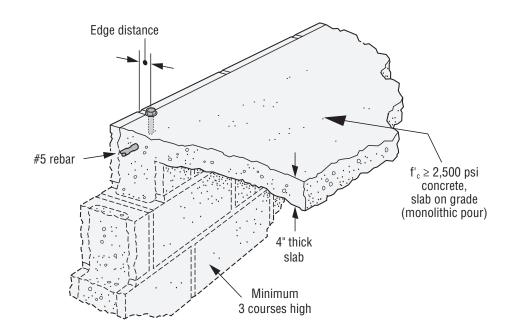
	0				0			
Size in. (mm)	Drill Bit	Minimum Embedment	Minimum Edge	Critical Spacing	8" Concrete-Filled CMU Chair Block Allowable Tension Loads Based on CMU Strength			
	Diameter (in.)	Depth in. (mm)	Distance in. (mm)	in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)		
¾ 8 ⅔⁄8 (9.5) ⅔⁄8	2¾ (60)	1¾ (44)	9½ (241)	3,175 (14.1)	635 (2.8)			
	3⁄8	3¾ (86)	1 ¾ (44)	13½ (343)	5,175 (23.0)	1,035 (4.6)		
		5 (127)	21⁄4 (57)	20 (508)	10,584 (47.1)	2,115 (9.4)		
1/2 (12.7) 1/2	1/	8 (203)	21⁄4 (57)	32 (813)	13,722 (61.0)	2,754 (12.2)		
	1/2	10 (254)	21⁄4 (57)	40 (1016)	16,630 (74.0)	3,325 (14.8)		
5%8 (15.9)	5⁄8	5½ (140)	1¾ (44)	22 (559)	9,025 (40.1)	1,805 (8.1)		

1. The tabulated allowable loads are based on a safety factor of 5.0.

2. Values are for 8"-wide concrete masonry units (CMU) filled with concrete, with minimum

compressive strength of 2,500 psi and poured monolithically with the floor slab.

3. Center #5 rebar in CMU cell and concrete slab as shown in the illustration below.



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Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads

How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- 4. Locate the edge distance (c_{act}) or spacing (s_{act}) at which the anchor is to be installed.
- 5. The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

Edge Dist	ance Tensi	on (f _c)				
	Dia.	1⁄4	3⁄8	1/2	5⁄8	3⁄4
	E	21⁄2	23⁄4	31⁄2	41⁄2	51⁄2
c _{act} (in.)	C _{cr}	4	12	12	12	12
()	C _{min}	1.25	4	4	4	4
	f _{cmin}	0.77	1.00	1.00	0.83	0.66
1.25		0.77				
2		0.83				
3		0.92				
4		1.00	1.00	1.00	0.83	0.66
6		1.00	1.00	1.00	0.87	0.75
8		1.00	1.00	1.00	0.92	0.83
10		1.00	1.00	1.00	0.96	0.92
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

c_{act} (in.)

Edge Distance Shear (f_c) Shear Load Parallel to Edge or End

1⁄4

21/2

4

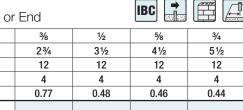
1 25

Dia.

Е

C_{cr}

c .



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	umin	1.20	4	4	4	4
	f _{cmin}	0.58	0.77	0.48	0.46	0.44
1.25		0.58				
2		0.69				
3		0.85				
4		1.00	0.77	0.48	0.46	0.44
6		1.00	0.83	0.61	0.60	0.58
8		1.00	0.89	0.74	0.73	0.72
10		1.00	0.94	0.87	0.87	0.86
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

Edge Distance Shear (f_c) Shear Load Perpendicular to Edge or End



1.E = embedment depth (inches).

2. c_{act} = actual end or edge distance at which anchor is installed (inches).

3. c_{cr} = critical end or edge distance for 100% load (inches).

4. c_{min} = minimum end or edge distance for reduced load (inches).

5. f_c = adjustment factor for allowable load at actual end or edge distance.

6. f_{ccr} = adjustment factor for allowable load at critical end or edge distance. f_{ccr} is always = 1.00.

7. f_{cmin} = adjustment factor for allowable load at minimum end or edge distance.

8. $f_c = f_{cmin} + [(1 - f_{cmin}) (C_{act} - C_{min}) / (C_{cr} - C_{min})].$

Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads (cont.)

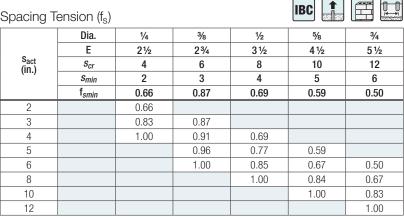
How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the embedment (E) at which the anchor is to be installed.
- 4. Locate the edge distance (cact) or spacing (sact) at which the anchor is to be installed.
- 5. The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- 6. Multiply the allowable load by the applicable load adjustment factor.
- 7. Reduction factors for multiple edges or spacings are multiplied together.

Edge Distance Shear (f_c) Shear Load Perpendicular to Edge or End (Directed Away From Edge or End)

	way i ion						
	Dia.	1⁄4	3⁄8	1⁄2	5⁄8	3⁄4	
	E	21⁄2	23⁄4	3 1/2	4 1⁄2	5½	
c _{act} (in.)	C _{Cr}	4	12	12	12	12	
()	C _{min}	1.25	4	4	4	4	
	f _{cmin}	0.71	0.89	0.79	0.58	0.38	
1.25		0.71					
2		0.79					
3		0.89					
4		1.00	0.89	0.79	0.58	0.38	
6		1.00	0.92	0.84	0.69	0.54	
8		1.00	0.95	0.90	0.79	0.69	
10		1.00	0.97	0.95	0.90	0.85	
12		1.00	1.00	1.00	1.00	1.00	

Spacing Tension (f_s)



IBC Ū Spacing Shear (f_s) Dia. 1⁄4 3⁄8 1⁄2 5⁄8 3/4 Е 21/2 2¾ 31⁄2 4 1⁄2 51⁄2 s_{act} (in.) s_{cr} 4 6 8 10 12 S_{min} 2 3 4 5 6 0.87 0.62 0.62 0.62 0.62 f_{smin} 2 0.87 3 0.93 0.62 4 1.00 0.75 0.62 5 0.87 0.72 0.62 6 1.00 0.81 0.70 0.62 8 1.00 0.85 0.75 10 1.00 0.87 12 1.00

1. E = embedment depth (inches).

2. s_{act} = actual spacing distance at which anchors are installed (inches).

3. s_{cr} = critical spacing distance for 100% load (inches).

4. s_{min} = minimum spacing distance for reduced load (inches).

5. f_s = adjustment factor for allowable load at actual spacing distance.

6. f_{SCT} = adjustment factor for allowable load at critical spacing distance. f_{SCT} is always = 1.00.

7. f_{smin} = adjustment factor for allowable load at minimum spacing distance.

8. $f_s = f_{smin} + [(1 - f_{smin}) (s_{act} - s_{min}) / (s_{cr} - s_{min})].$

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