

Architectural Products

STRAP TIES

Model No.	Ga.	Dimensions (in.)		Fasteners (in.)			Allowable Tension Loads (DF/SP)	Allowable Tension Loads (SPF/HF)
		W	L	Nails	Bolts		Bolts	Bolts
					Qty.	Dia.	(160)	(160)
HST2PC	7	2½	21¼	—	6	⅝	5,220	4,835
HST5PC		5	21¼	—	12	⅝	10,650	9,870
HST3PC	3	3	25½	—	6	¾	7,650	6,580
HST6PC		6	25½	—	12	¾	15,425	13,265

1. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: HST2PC and HST5PC — 3 1/2"; HST3PC and HST6PC — 4 1/2".

BEAM-TO-COLUMN TIES

Model No.	Ga.	Dimensions (in.)			Minimum Bolt End and Edge Distances (in.)		Bolts		Allowable Loads ^{1,2}	
		W	H	L	d ₁	d ₂	Qty.	Dia.	Tension/Uplift	F ₁
									(100/160)	(100/160)
1212HLPC	8	2½	12	12	2½	4⅜	5	⅝"	1,650	725
1616HLPC	8	2½	16	16	2½	4⅜	5	⅝"	1,650	725

1. 1212HLPC and 1616HLPC are to be installed in pairs with machine bolts in double shear. A single part with machine bolts in single shear is not load rated.
2. Allowable loads are based on a minimum member thickness of 3 1/2".

Model No.	Ga.	Dimensions (in.)			Minimum Bolt End and Edge Distances (in.)		Bolts		Allowable Loads ^{1,2}	
		W	H	L	d ₁	d ₂	Qty.	Dia.	Tension/Uplift	F ₁
									(100/160)	(100/160)
1212HTPC	8	2½	12	12	2½	4⅜	6	⅝"	2,380	915
1616HTPC	8	2½	16	16	2½	4⅜	6	⅝"	2,380	915

1. 1212HTPC and 1616HTPC are to be installed in pairs with machine bolts in double shear. A single part with machine bolts in single shear is not load rated.
2. Allowable loads are based on a minimum member thickness of 3 1/2".
3. 1212HTPC and 1616HTPC loads assume a continuous beam.

COLUMN BASES

Model No.	Ga.	Dimensions (in.)		Bolts		Allowable Tension Loads (160)
		W ₁	W ₂	Qty.	Dia.	
CB44PC	7	3 $\frac{3}{8}$ "	3 $\frac{1}{2}$ "	2	$\frac{5}{8}$ "	4,200
CB46PC	7	3 $\frac{3}{8}$ "	5 $\frac{1}{2}$ "	2	$\frac{5}{8}$ "	4,200
CB48PC	7	3 $\frac{3}{8}$ "	7 $\frac{1}{2}$ "	2	$\frac{5}{8}$ "	4,200
CB66PC	7	5 $\frac{1}{2}$ "	5 $\frac{1}{2}$ "	2	$\frac{5}{8}$ "	4,200
CB68PC	7	5 $\frac{1}{2}$ "	7 $\frac{1}{2}$ "	2	$\frac{5}{8}$ "	4,200
CB88PC	3	7 $\frac{1}{2}$ "	7 $\frac{1}{2}$ "	2	$\frac{3}{4}$ "	6,650
CB810PC	3	7 $\frac{1}{2}$ "	9 $\frac{1}{2}$ "	2	$\frac{3}{4}$ "	6,650

1. Allowable loads have been increased 60% for wind or earthquake loading with no further increase allowed; reduce where other loads govern.
2. Minimum side cover for full loads is 3" for CBs.
3. Install with bottom of base flush with concrete.
4. Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

COLUMN CAPS

Model No.	Ga.	Dimensions (in.)				Bolts				Allowable Loads	
						Beam		Post		Uplift	Down
		W ₁	W ₂	L	H	Qty.	Dia.	Qty.	Dia.	(160)	(100)
CC44PC	7	3 $\frac{3}{8}$ "	3 $\frac{3}{8}$ "	7	4	2	$\frac{5}{8}$ "	2	$\frac{5}{8}$ "	1,465	15,310
CC46PC	7	3 $\frac{3}{8}$ "	5 $\frac{1}{2}$ "	11	6 $\frac{1}{2}$ "	4	$\frac{5}{8}$ "	2	$\frac{5}{8}$ "	2,800	24,060
CC66PC	7	5 $\frac{1}{2}$ "	5 $\frac{1}{2}$ "	11	6 $\frac{1}{2}$ "	4	$\frac{5}{8}$ "	2	$\frac{5}{8}$ "	4,040	30,250
CC68PC	7	5 $\frac{1}{2}$ "	7 $\frac{1}{2}$ "	11	6 $\frac{1}{2}$ "	4	$\frac{5}{8}$ "	2	$\frac{5}{8}$ "	4,040	37,810
CC88PC	3	7 $\frac{1}{2}$ "	7 $\frac{1}{2}$ "	13	8	4	$\frac{3}{4}$ "	2	$\frac{3}{4}$ "	7,440	54,600

1. Allowable loads have been increased 60% for wind or earthquake loading with no further increase allowed; reduce where other loads govern.
2. Post sides are assumed to lie in the same vertical plane as the beam sides.
3. Downloads are determined using F'c perpendicular equal to 625 psi on seat area; reduce where end bearing value of post, L/R of post, or other criteria are limiting.
4. See ECC/ECCU for glulam beam sizes and conditions. Add PC to the model, i.e. CC31/4-4PC.
5. Column caps for end conditions available to order, add an "E" to the start of the model number. See ECC/ECCU Load Table for load values.