

GP — Gusset Plate



Prepunched Holes and Dimples Allow For Easy Installation

The GP gusset plate is designed to secure cold-formed steel (CFS) strapped shearwalls, truss connectors, miscellaneous wall connectors and other framing applications. The GP66 (6" x 6") and GP69 (6" x 9") have holes conveniently located 1" center-to-center to make screw installation easy and quick. The GP612 (6" x 12") and GP1212 (12" x 12") have holes on the outside edges and dimples in the center. The dimples act as screw locators when attaching from gusset plate to CFS member. When attaching from CFS member (i.e., strap bracing) to gusset plate, the dimples avoid the interference that prepunched holes could cause.

Features

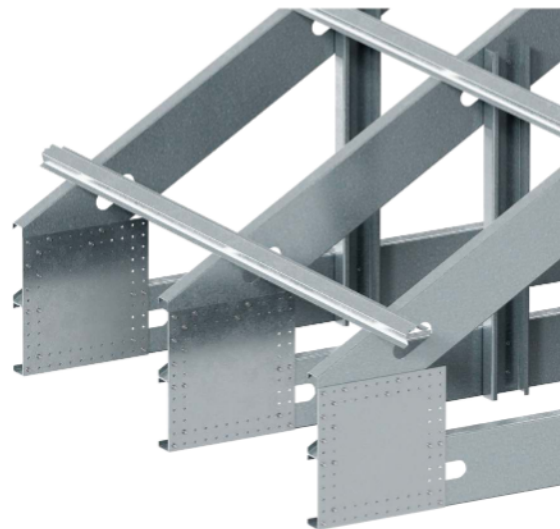
- Multiple stocked sizes and thicknesses to meet job site needs
- Prepunched holes for easy and quick installation
- Dimples in the GP612 and GP1212 models provide more flexibility for fastening cross-bracing to the gusset plate as they act as screw placement locators that can accommodate various strap and framing dimensions

Material: 43 mil (18 ga.) — 33 ksi; 54 mil (16 ga.) — 50 ksi;
68 mil (14 ga.) — 50 ksi; 97 mil (12 ga.) — 50 ksi

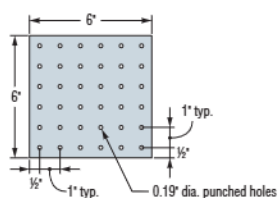
Finish: Galvanized (G90)

Ordering Information:

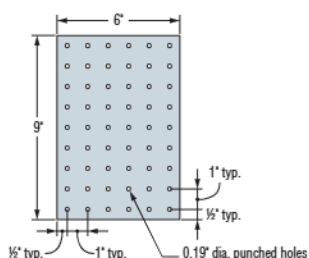
Model No.	Ordering SKU	Size (in.)	Thickness mil (ga.)	Package Qty.
GP66/54	GP66/54-R50	6 x 6	54 (16)	50
GP69/43	GP69/43-R30	6 x 9	43 (18)	30
GP69/54	GP69/54-R25		54 (16)	25
GP612/68	GP612/68-R20	6 x 12	68 (14)	20
GP1212/68	GP1212/68-R15	12 x 12	68 (14)	15
GP1212/97	GP1212/97-R10		97 (12)	10



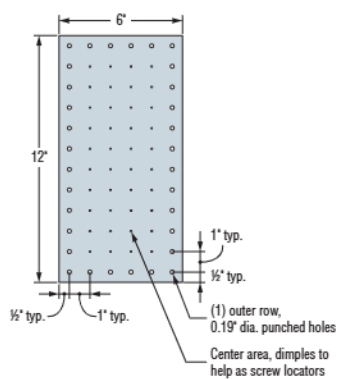
6" x 6" (GP66/54)



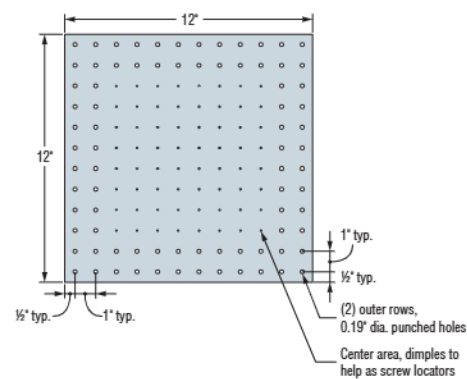
6" x 9" (GP69/43, GP69/54)



6" x 12" (GP612/68)



12" x 12" (GP1212/68, GP1212/97)



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Design Example

Given:

- 2024 IBC and AISI S100-16
- 600S162-68 (50 ksi) studs
- 600T125-68 (50 ksi) tracks @ top and bottom
- Gusset Plate: GP1212-68 ($F_y = 50$ ksi, $F_u = 65$ ksi)
- Strap 6" wide ($F_y = 50$ ksi, $F_u = 65$ ksi) — 14 ga. (0.0713" design thickness)

- Wall height, $H = 10'$; Wall width, $W = 8'$
- Horizontal wind load at top of wall from diaphragm = 8,000 lb. (ASD)
- The straps install on both side of the wall; screw spacing is specified as 1" in both horizontal and vertical directions

Calculations:

The wind load is $V = 8,000$ lb. (4,000 lb./ea. side). Summing the moments about A to achieve equilibrium, we have the following expression. *Note axial load has been neglected because this assumption produces the highest anchor tension demand:*

$$\Sigma M_A = V_H \times H - R_y \times W = 0$$

Determine ASD Vertical, Horizontal and Diagonal Loads:

Vertical chord load =	10,000 lb. (5,000 lb. ea. GP plate)
Horizontal chord load =	8,000 lb. (4,000 lb. ea. GP plate)
Diagonal load =	6,403 lb. ea. strap

Flat Tension Strap:

• For strap yielding:

$$\begin{aligned} T_n &= A_g F_y & (\text{Eq. D2-1}) \\ \text{Gross area of cross-section } A_g &= 0.428 \text{ in}^2 \\ \Omega_t &= 1.67 \\ T_n / \Omega_t &= 12,808 \text{ lb.} \end{aligned}$$

• For fracture away from connection:

$$\begin{aligned} \text{Assume 3 screw holes: } d_{\text{hole}} &= 0.190 \text{ in.} \\ T_n &= A_n F_u & (\text{Eq. D3-1}) \\ \text{Net area of cross-section } A_n &= 0.387 \text{ in}^2 \\ \Omega_t &= 2.0 \\ T_n / \Omega_t &= 12,583 \text{ lb.} \end{aligned}$$

• Tension Rupture:

$$\begin{aligned} \text{Effective net area subject to tension } A_e &= U_{sl} A_{nt} & (\text{Eq. J6.2-1}) \\ \text{Shear lag factor } U_{sl} &= 0.910 & (\text{Eq. J6.2-4}) \\ \text{Net area subject to tension } A_{nt} &= 0.387 \text{ in}^2 \\ \Omega &= 3 & (\text{Table J6-1}) \\ P_{nt} / \Omega &= 7,629 \text{ lb.} \\ \text{Allowable Tension of Strap, } T_{\text{allow}} &= 6,403 \text{ lb.} \quad \text{OK} \end{aligned}$$

Tension capacity of Gusset Plate:

• For Gusset Plate yielding:

$$\begin{aligned} T_n &= A_g F_y & (\text{Eq. D2-1}) \\ \text{Gross area of cross-section } A_g &= 0.612 \text{ in}^2 \\ \Omega_t &= 1.67 \\ T_n / \Omega_t &= 18,318 \text{ lb.} \end{aligned}$$

• For fracture away from connection:

$$\begin{aligned} \text{Assume 6 screw holes: } d_{\text{hole}} &= 0.190 \text{ in.} \\ T_n &= A_n F_u & (\text{Eq. D3-1}) \\ \text{Net area of cross-section } A_n &= 0.530 \text{ in}^2 \\ \Omega_t &= 2.0 \\ T_n / \Omega_t &= 17,213 \text{ lb.} \end{aligned}$$

• Tension Rupture:

$$\begin{aligned} \text{Effective net area subject to tension } A_e &= U_{sl} A_{nt} & (\text{Eq. J6.2-1}) \\ \text{Shear lag factor } U_{sl} &= 0.913 & (\text{Eq. J6.2-4}) \\ \text{Net area subject to tension } A_{nt} &= 0.530 \text{ in}^2 \\ \Omega &= 3 & (\text{Table J6-1}) \\ P_{nt} / \Omega &= 10,482 \text{ lb.} \\ \text{Allowable Tension of Gusset Plate, } T_{\text{allow}} &= 6,403 \text{ lb.} \quad \text{OK} \end{aligned}$$

#10 screws 14 ga. (50 ksi) Strap attached to 14 ga. (50 ksi) Gusset Plate

ASD Shear #10 Screws 540 lb. (Simpson Strong-Tie® #10 X Metal Screw)

Strap-to-Gusset Plate Connection:	Number of screws required = 11.86	Use 12 #10 screws
Gusset-to-Stud Connection:	Number of screws required = 9.26	Use 10 #10 screws
Gusset-to-Track Connection:	Number of screws required = 7.41	Use 8 #10 screws

